GEOTECHNICAL ASSESSMENT TAILINGS OPERATIONS AND STORAGE

PROJECT: BARRYTOWN MINERAL SANDS TAILINGS

TIGA MINERAL & METALS LTD



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# 1 INTRODUCTION

TiGa Minerals & Metals Ltd (TiGa) engaged Resource Development Consultants Ltd (RDCL) to provide geotechnical services to assess geotechnical aspects of the proposed mining and tailings storage for the Barrytown Sands Project at Barrytown, Grey District, New Zealand.

Mining will be by open pit to  $\sim$ 14m below current ground level, and will be undertaken with excavators, mine trucks and dozers. It is anticipated that mining will be complete within 4 – 5 years.

An overview of mining geotechnical processes that could affect excavation stability was requested for inclusion with a Resource Consent Application.

# 1.1 SCOPE

Our scope has been to assess geotechnical aspects of:

- Mining Operations including stability of excavation and tailings operations; and
- Tailings storage assessment and risk assessment.

Other professional organisations involved relative to this work include:

- Subterra Ltd providing geotechnical services;
- Palaris Pty Ltd providing mining engineering services;
- RSC Consultants Ltd providing geological services; and
- Kōmanawa Solutions Ltd providing hydrogeological services.



#### 2 **GUIDELINES, STANDARDS AND LEGISLATIVE REQUIREMENTS**

Operations, Storage and Closure of the proposed tailings facility has been assessed considering National requirements and International Guidelines including:

- Resource Management Act 1992; •
- NZ Health and Safety at Work Act 2015;
- NZSOLD (2015) Design Guidelines for Large Dams
- Global Industry on Tailings Management (International Council on Mining and • Metals et al., 2020);
- ANCOLD (2022) Recommendations for Tailings Dams; •
- ICOLD 220131 Tailings Dam Safety Bulletin; and •
- CDA (2021) Technical Bulletin on Tailings Dam Breach Analysis.



#### 3 **PROJECT DESCRIPTION**

#### 3.1 **OVERVIEW**

The proposed project is to extract Heavy Mineral sands contained within the Holocene Beach Placer deposit at Barrytown (Figure 1). The project geology, mining, processing and tailing storage are not complicated from a geotechnical perspective; also being without the use of chemical additives in the processing stream.

The site is within existing, modified farmland, sloping from SH6 west towards the coast, with elevation difference of ~15m across the project footprint. Wetlands border the property to the south and west, with a small drainage channel on the northern boundary and Collins Creek to the south.

#### 3.2 **RESOURCE AND PRODUCTION**

The resource estimates is for  $\sim$ 4,800,000 tonnes of recoverable sand ore mined over 5 – 7 years for annual production rate ~250,000 tonnes of Heavy Mineral Concentrate for 1,100,000 tonnes total mined.



#### **GEOLOGICAL SETTING** 4

#### 4.1 **REGIONAL GEOLOGY**

Summary geology in this report is taken from RSC (2022) which includes a detailed description of the regional and project geology, and which should be referred.

## 4.1.1 PUBLISHED GEOLOGY

For the geotechnical design and risk assessment the geological aspects of the project are:

- Mineralisation occurs as marine placer deposits of heavy minerals, concentrated by longshore drift and pushed up by wave action (Figure 2).
  - Lensoidal shaped mineral concentrations follow the dip of the beach towards the sea at  $\sim 5^{\circ}$  to  $10^{\circ}$ ;
  - Deposits are generally very well sorted fine sand, with occasional clay and gravel intercalations.
  - Maximum depth of deposit is ~14m. -
- Overburden comprises Clayey, silty Gravel derived broadly from colluvial outwash; •
- Basement comprises barren Gravel with the contact being abrupt and comprising a wave cut platform.

#### 4.2 GROUNDWATER

Groundwater levels are assumed 1 to 2m below current ground level.

#### 4.3 **GEOHAZARDS**

### 4.3.1 ACTIVE FAULTS

No active faults directly impacting the site have been identified in the New Zealand Active Faults Database (GNS Science, 2018).



The Canoe Fault (Figure 1) is oriented striking north-east and is ~1km east of the site at the centre of the block, and classified with:

- Reverse fault; with
- Holocene; and
- Unknown displacement and slip rate.

# 4.3.2 LIQUEFACTION RISK

Grey District Council (GDC) does not cover liquefaction potential but shows Barrytown flats with an elevated ground shaking risk reflecting the sandy nature of the site.

Investigations for this project study (Subterra 2022) indicate surface clays are unlikely to liquefy, and that a non-liquefiable layer 4m thick is likely across the site at depths from 6m - 8m mbgl.

# 4.3.3 SUMMARY OF GEOHAZARDS

Geohazard	Risk Level	Risk Summary
Active Faults	Low	No "known" active faults directly impacting the site
Liquefaction Susceptibility	Moderate	The property is underlain by fine sand and is likely susceptible to some form of liquefaction.
Ground Shaking	High	The property is within a 'high' amplification area – susceptible to ground shaking in an earthquake.
Slope Stability	Low	The site is not at risk of inundation by landslide.
Coastal Interaction	Low	The mining block is separated from wave action by the adjacent lagoon.
Tsunami	Low	The site is at low risk of tsunami hazard.

### TABLE 1: SUMMARY OF GEOHAZARDS



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#### 5 **MINING METHOD**

#### 5.1 SUMMARY

The main elements of the mining process include (Figure 3):

- Preparatory works prior to mining to establish including but not limited to:
  - Overland water diversion drains,
  - Visual bund;
  - 20m offset established; and
  - Water Facility.
- Mining by excavator to ~14m depth below existing ground level with:
  - Barren overburden stripped in advance of the mining face; and -
  - Mineralised sands fed to a mobile hopper;
  - Oversize and slimes removed at point of mining;
  - Mineralised slurry transported by pipeline; to the
- Processing plant with:
  - Mineral fraction separated by gravity using no chemical additives;
  - Waste sand fraction (tailings) returned to pit void as a slurry transported by pipeline; with
- Tailings will be: •
  - Dewatered by cyclone at the discharge point to the mine void;
  - Pushed out by bulldozer within the pit; and \_
- Final landform formed as the mine advances by:
  - Overburden placed over to cap the tailings;
  - Shaped by mechanical (excavator, tractor, truck etc) means; \_
  - Top soiled and returned to farming.



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5.2 MINING PLAN

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The active mine void will cover 3 Ha in a 300m long x 100m wide strip. Over burden and topsoil will be stripped in advance of mining in 0.5Ha increments using an excavator. Overburden will be trucked and placed at the back of the mining void for use in progressive rehabilitation.

#### 5.2.1 MINING

Ore (mineral sands) will be mined by 80T excavator from a  $\sim$ 1 Ha bench where the mining and desliming field units will be located on skids. The rate of mining advance will be approximately 5 metres per day (35 metres per week).

1 Ha of the mining void will be actively receiving tailings pumped from the processing plant. Tailings are dewatered and discharged to the mining void via cyclone. The tailings will be allowed to naturally beach out (spread out). The cyclone will be moved as required to distribute the tailings as necessary.

Mining will be undertaken during daylight hours only and provide enough material to enable 24 hour operation of the processing plant (i.e. The rate of mining is approximately double the rate of processing). Tailings will be pumped to the mining void at night.

The open pit will be excavated to ~14m below existing ground with temporary slopes at  $50^{\circ}$ - $60^{\circ}$ .

#### 5.2.2 TAILINGS

Tailings will be levelled and contoured with the use of excavators and bulldozers ready to receive the pre stripped overburden and soil as out lined in point 1 of this section. The mining void will be progressively rehabilitated as the mining void advances.

#### 5.2.3 REHABILITATION

Vegetative cover (sowing of grass) is established, and the area is removed from the disturbed area once 80% vegetative cover is achieved.

#### 5.2.4 TIME FRAME

Each mining panel will take between 4 and 6 months to be mined and rehabilitated. Depending on volume of ore and weather conditions during rehabilitation.



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#### 6 TAILINGS OPERATION

Tailings are proposed to be discharged into the mining void being dewatered by cyclone at that point.

Dry tailings will be pushed out into the void by bulldozer, advancing 5m / day working within a 25m strip from the completed surface and with no operator entry into the pit.

#### 6.1 TAILINGS STORAGE

Tailings:

- Are "clean" with no toxic potential; •
  - Liners for containment are not required. -
- Will be stored in the mine void which is:
  - Below ground level; with \_
  - No enclosing embankments; and \_
  - Freeboard always > 3m to ground level from final tailings surface as ~30% of \_ the material is extracted as ore.
- Will be capped using overburden concurrent with mining advance, with the final surface finished to allow farming.

Tailings storage void:

- Is expected to be stable from a geotechnical perspective; with •
- Free of active faults: with
- Low potential for liquefaction; and •
- Rehabilitation plan to protect against overland water flows.



# 7 GEOTECHNCIAL ASSESMENT

# 7.1 GROUND MODEL

### 7.1.1 PROCESSING PLANT

A geotechnical drilling program was carried out for the processing plant site location (Subterra, 2022). The typical geotechnical ground profile from that work is in Table 2.

Depth to base of unit (m bgl)	Material	Density / Consistency	SPT (N <sub>60</sub> )
1.7 to 2.2	CLAY	Soft	
5.9 to 8.0	CLAY with variable organic content	Soft to firm	
At least 11.4	SAND with variable gravel	Medium Dense to Dense	

TABLE 2 PROCESSING PLANT GEOTECHNICAL GROUND MODEL (SUBTERRA, 2022)

# 7.1.2 MINING BLOCK

Within the mining block, the ground model is based on the geological section inferred from exploration drilling (Figure 4) with type sections in Figure 2.

The mining block ground model is in Table 3.

TABLE 3 MINING	BLOCK GROUND	

Geotechnical Unit	Depth (From-To)	Material
Overburden	0-2	Clay, Silt and Gravel
Mineralised Sands	2-14	Sand, fine, well sorted
Barren Gravel Base	14-17	Gravel



#### 7.2 **MATERIAL GEOTECHNICAL PROPERTIES**

### 7.2.1 LABORATORY TESTS

Laboratory tests have been undertaken on exploration sample splits (Figure 4) including:

- 11 x Particle size distribution (Plate 2); •
- 11 x Compaction tests (Table 2); and
- 9 x Atterberg Limit Tests (Table 2). •

Laboratory tests results are in Appendix A.

#### **TABLE 4: LABORATORY TEST RESULTS**

Sample	Material (Laboratory Description)	Dry Density (t/m <sup>3</sup> )	Natural WC (%)	Bulk Density at OWC (t/m <sup>3</sup> )	OMC (%)
TAC007	Silt SAND with minor gravel and trace clay	1.89	0.5	2.16	14.0
TAC035	Silty SAND with trace gravel and clay	1.70	0.2	1.91	12.0
TAC070	Silty SAND with trace gravel and clay	1.96	0.1	2.19	12.0
TAC097	SAND with some silt, trace gravel and clay	1.84	0.2	2.02	11.0
TAC153	SAND with some gravel, minor silt and trace clay	2.24	0.2	2.43	8.0
TAC170	Gravelly SAND with some silt and trace clay	2.14	0.3	2.27	6.0
TAC176	SAND with minor silt, trace gravel and clay	1.75	0.0	1.97	12.0
TSON002	SAND with some gravel minor silt and trace clay	2.05	0.1	2.26	10.0
TSON003	SAND with some gravel and minor silt	1.88	0.2	2.04	10.0
TSON004	SAND with some gravel, silt and trace clay	1.80	0.1	2.06	14.0

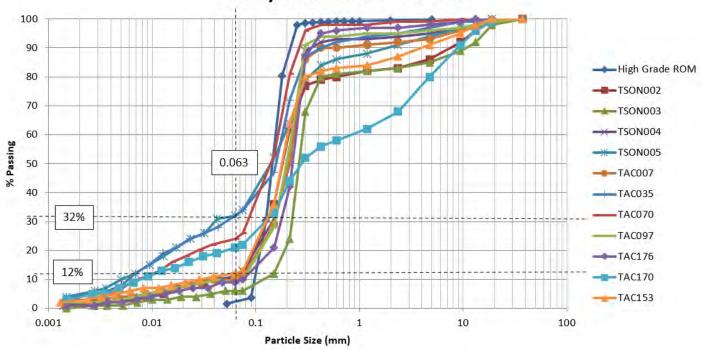


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Sample	(Laboratory Description)		Natural WC (%)	Bulk Density at OWC (t/m <sup>3</sup> )	OMC (%)
TSON005	Silty SAND with minor gravel and trace clay	1.94	0.2	2.18	12.0

#### PLATE 1: PARTICLE SIZE DISTRIBUTION CURVES



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# 7.2.2 GEOTECHNICAL PARAMETERS FOR DESIGN

Material and deformation properties assumed for geotechnical modelling (Table 1) are based on assessment of:

- Discussion with exploration geologists (RSC) and hydrogeologists (Kōmanawa) who attended drilling at the site;
- Laboratory tests; and
- Engineering correlations from published sources (Look, 2007).



Geotechnical Unit	Material	Depth (From-To)	Friction (°)	Cohesion (c', kPa)	Unit Weight (kN/m²)	Permeability (m/s)
Overburden	Clay, Silt and Gravel	0-2	30	2	15	10-6
Mineralised Sands	Sand, fine, well sorted	2-10	40	-	18	10-4
Barren Gravel Base	Gravel	10-17	42	-	20	10-7
Tailings	Sand, fine, well sorted, heavy metals removed	3-10	30		18	10 <sup>-5</sup>

#### TABLE 2: GEOTECHNICAL PARAMETERS

### 7.3 SEISMIC DESIGN

Seismic stability has been checked against:

- ICOLD 220131criteria; using
- Christchurch 2011 seismic event captured from station located at Christchurch Hospital (CHHC) scaled to the ground accelerations developed by MBIE Module 1 2021.

### 7.3.1 SEISMIC SOIL CLASSIFICATION

The site subsoil class is assumed "Class D Deep Soil" in accordance with NZS1170.5:2004, part 5: Earthquake Actions – New Zealand. The site subsoil class was assessed based on regional geology.



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# 7.3.2 SEISMIC LOADS BASIS OF ASSESSMENT

Seismic design loads for a Low PIC dam (Section 8.3) are:

- Operating Base Earthquake (OBE): 150 year return period; and
- Maximum Credible Earthquake (MCE): 500 year return period.

Assessment of seismic loads has been undertaken using MBIE Guidelines Module 1, Overview of Earthquake Geotechnical Engineering Practice Guidelines, (2021, version 1) as appropriate for a Low PIC dam:

- Magnitude (M) = 6.5 (OBE) & 6.7 (MCE);
- Peak Ground Acceleration (PGA) = 0.30g (OBE) & 0.53g (MCE)

#### 7.4 **STABILITY ANALYSIS**

### 7.4.1 MINING OPERATIONS AND TAILINGS STORAGE

Stability analyses has been undertaken using limit equilibrium to assess the Factor of Safety against failure for the:

- Initial Pit Excavation: and
- Initial Pit Excavation + Tailings backfill (initial placement only). •

Finite Element Methods have also been undertaken to demonstrate deformation of the tailings storage facility:

- Under seismic loads (both OBE & MBE); and
- Considering liquefaction potential.

Analyses has been undertaken for cases:

- Initial Pit Excavation: Mining to ~14m depth; and
- Initial Pit Excavation + Tailings backfill: Tailings placed at 3 m from ground level.

Analytical outputs for all stages are in Appendix B.



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# 7.4.2 LIMIT EQUILIBRIUM FACTOR OF SAFETY

- Limit equilibrium analyses has been used to assess factor of safety (FoS) using: •
  - Program Slide2 v9.009 being widely accepted software by Rocscience; and \_

Derived Factors of Safety are compared with the ICOLD 220131criteria (Table 3).

All conditions meet or exceed the stability criteria.

### TABLE 35: LIMIT EQUILIBRIUM ASSESSMENT RESULTS

Stage	Load Condition	Factor of Safety	Criteria to Meet
0	Static	>1.5	1.5
Open Pit Cut Slope	OBE (0.3g)	>1.1	1.1
	MCE (0.53g)		. –

# 7.4.3 **DEFORMATION ANALYSES**

Seismic induced deformation analyses have been undertaken using FEM (RS2) to provide an indication of deformation associated with the scaled (to 0.3 and 0.53g) earthquake load (Appendix B, Table 4).

Deformation is estimated at the 20m setback boundary.

### **TABLE 46: DEFORMATION ASSESSMENT RESULTS**

Stage	Load Condition	Total Deformation (m)
	Static	< 0.08m
Open Pit Cut Slope	OBE (0.3g)	0.4
	MCE (0.53g)	0.5
	Static	0.02
With Tailings Backfill	OBE (0.3g)	0.1
	MCE (0.53g)	0.2



# 8 MINING OPERATION AND TAILINGS

Key aspects of the mining operation from a geotechnical perspective are:

- Stability of initial pit excavation;
- Drainage potential of tailings for operating a bulldozer to achieve the proposed mining advance rate; and
- Safety in design.

# 8.1 STABILITY OF PIT SLOPES

The proposed initial pit slope will be excavated at 50°-65° to a depth up to 14m below original ground surface.

This temporary slope at the outer limit of the mining block will be buttressed as tailings are returned into the mine void; in effect the critical slope will be left unsupported for days to weeks only.

Slope stability numerical assessment (Section 6.4.1 and 6.4.2) indicate acceptable factors of safety against failure for the proposed initial cut considering all cases including seismic (~MCE) conditions.

### 8.2 DRAINAGE POTENTIAL AND MINING ADVANCE

The drainage potential of the tailings is an important consideration in mine advance as a bulldozer is proposed to push the tailings into the mine void with mining advance. The tails must be drained to allow the bulldozer onto the tailing surface without becoming stuck. Tailings advance is proposed at 5m / day to keep up with mining.

The permeability (and hence drainage potential) of the tailings has been assessed based on particle size distribution also considering Atterberg limits as being commonly available test in New Zealand. Specialised test to assess tailings material properties are otherwise not available in the country.

Representative material testing for estimating drainage potential of tailings was from Bulk Samples taken from exploration drill composites with 1 no. x ROM sample.



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### 8.2.1 PARTICLE SIZE DISTRIBUTION

- Bulk Sample is very well sorted fine sand with:
  - Fines fraction (slimes; < 0.063mm) < 12% in 7 holes.
  - Fines fraction 20% to 32% in 4 holes.
- ROM Sample is very well sorted fine sand with:
  - Fines fraction (< 0.063mm) < 4%.

#### 8.2.2 INFERRED PERMEABILITY

The Inferred Permeability of Bulk Sample is:

- $\sim 10^{-5}$  to  $10^{-4}$  m/s for samples with Fines content < 12%;
- $\sim 10^{-7}$  m/s for the samples with Fines < 32%.
- Based on published correlations as in Table 5.



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Soil type	D	escription	k, m/s	Drainage		
Cobbles and boulders	Flow may be turbulent	, Darcy's law may not be valid	$ \begin{array}{c} 10^{-1} \\ 10^{-2} \\ 10^{-3} \\ 10^{-4} \\ 10^{-5} \\ 10^{-6} \\ 10^{-7} \\ \end{array} $			
Gravels	Coarse Clean	Uniformly graded coarse aggregate		10-2	Very good	
Gravel sand mixtures	Clean	Well graded without fines		10-4	10-4	
Sands	Clean, very fine Silty Stratified clay/silts	Fissured, desiccated, weathered clays Compacted clays – dry of		Good		
Silts	Homogeneous below zone of weathering	optimum	10 <sup>-8</sup> 10 <sup>-9</sup> 10 <sup>-10</sup>	Poor		
Clays		Compacted clays – wet of optimum	10 <sup>-11</sup> 10 <sup>-12</sup>			
Artificial	Bituminous, cements stabilized soil Geosynthetic clay liner / Bentonite enriched soil concrete			Practically impermeable		

TABLE 5 INFERRED PERMEABILITY FROM PARTICLE SIZE DISTRIBUTION TESTS (FROM LOOK, 2007)

Granular material is no longer considered free draining when the fines >15%. .

Granular material is often low permeability (if well compacted) when the fines >30%.

Table 8.3 Permeability based on Hazen's relationship.

Coarse grained size	>Fine	>Fine sands		>Medium sands				>Coarse sands			
Effective grain size d <sub>10</sub> ,mm	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0	
Permeability ( $k = Cd_{10}^2$ )	10 <sup>-4</sup> m/s		10 <sup>-3</sup> m/s				10-		10-2	² m/s	
C = 0.10 (above equation)	I.	4	0.9	1.6	2.5	3.6	4.9	6.4	0.8	1.0	
C=0.15	1.5	6	1.4	2.4	3.8	5.4	7.4	9.6	1,2	1.5	

#### 8.2.3 CLAY REACTIVITY

All samples are non-plastic with no reactivity based on linear shrink tests.

For the clay fraction testing indicates materials are:

- Non plastic; and
- Not susceptible to Linear shrinkage; being generally •
- "Not reactive".



#### 8.2.4 GEOTECHNICAL ASSESSMENT OF TAILINGS ADVANCE

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We believe the mining cycle demand requiring 5m strip of tails to be pushed back into final landform can be achieved. This is based on test results taken for this assessment.

We understand that the tails will be discharged into the mine pit through a cyclone. The sand is likely to beach at (allow) 27° if discharged wet, steepening with drying, with the clay (slimes) fraction running out into the gap between the tails and mining face.

We believe the sand fraction will drain relatively quickly to allow handling into the final landform to meet the mining schedule. The clay fraction will take longer to dry but is a minor fraction in all cases so is not expected to drive the sequence.

We do not anticipate the drainage time to be significantly affected if the slimes are mixed into the tails at the point of discharge given the low fines fraction in most samples.

For the samples with higher slimes content (to 30%), at this stage we would anticipate the material would be blended through the mining cycle and that the resulting slimes content would not increase over say 15%. That would be unlikely to significantly affect the material drainage properties.

#### 8.3 SAFETY IN DESIGN

NZ Health and Safety at Work Act (2015) requires consideration of Safety in Design with Section and Mine Safety Regulations. For the proposed mining method, principal hazards are:

- Stability of temporary slopes;
- Tailings inundation; and
- Flooding in the pit floor.

It is proposed to restrict personnel entry into the mining void, reducing exposure to the principal hazards and adequately mitigating risk.

### 8.4 ENVIRONMENTAL RISK DUE TO STABILITY AND COASTAL EROSION

### 8.4.1 OPEN PIT STABILITY

The open pit is expected to be stable for the proposed configuration with no substantial ground displacement due to instability expected > 5m from the pit crest based on this study.



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The coastal lagoon, Collins Creek, Northern Drain and property boundaries are at low to very low risk of being adversely affected due to mining during operations and for the finished landform.

# 8.4.2 COASTAL INTERACTION

The rehabilitated ground will be made up of hydraulically and mechanically placed tailings overlain by a clay cap placed and compacted by machines including oversize, finished to pasture for dairy use.

The resulting "engineered landform" is considered resilient from a geotechnical perspective considering earthquake, coastal process and weather.

There is a low to very low risk of adverse interaction with coastal processes for the proposed finished landform.



#### 9 TAILINGS STORAGE

#### 9.1 **OVERVIEW**

Tailings are proposed to be stored in the mining void. Tailings are:

- Are "clean" with no toxic potential;
  - Liners for containment are not required.
- Will be stored in the mine void which is:
  - Below ground level; with
  - No enclosing embankments; and -
  - Freeboard always > 3m to ground level from final tailings surface as  $\sim$ 30% of \_ the material is extracted as ore.
- Will be capped using overburden concurrent with mining advance, with the final surface finished to allow farming.

#### 9.2 SITE ASSESSMENT

The geological setting including understanding of potential hazards is well known and meets the criteria for RMA (2015) Preliminary Geotechnical Assessment and the Global Industry on Tailings Management Topics 2 & 3 being relevant to this level of assessment.

#### 9.3 NZSOLD PIC

The tailing storage facility is with Low Potential Impact Classification (PIC) in accordance with NZOLD (2015) Guidelines.

#### 9.4 **STABILITY**

The stability of the tailings facility being within the mine void has been assessed:

- Considering seismic loads and liquefaction potential; •
- Using limit equilibrium and Finite Element Methods (Section 6.4.2 & 6.4.3).



In all cases:

- Acceptable Factors of safety are achieved; and
- Deformation under earthquake loads and potential liquefaction is limited with no breach in freeboard likely.

# 9.5 RISK ASSESSMENT

A Potential Failure Mode Analysis (PFMA) is presented (Appendix C) focusing on geotechnical aspects of the tailing storage facility.

The work has been done in general accordance with:

• CDA (2019) Technical Bulletin on Tailings Dam Breach Analyses. Authors Martin, V et al Al-Mamun, M & Small, A.

Considering:

- NZSOLD (2015) Design Guidelines for Large Dams;
- Global Industry on Tailings Management (International Council on Mining and Metals et al., 2020);
- ANCOLD (2022) Recommendations for Tailings Dams;
- ICOLD 220131 Tailings Dam Safety Bulletin.

### 9.6 POTENTIAL FAILURE MODE ANALYSES

#### 9.6.1.1FRAMEWORK

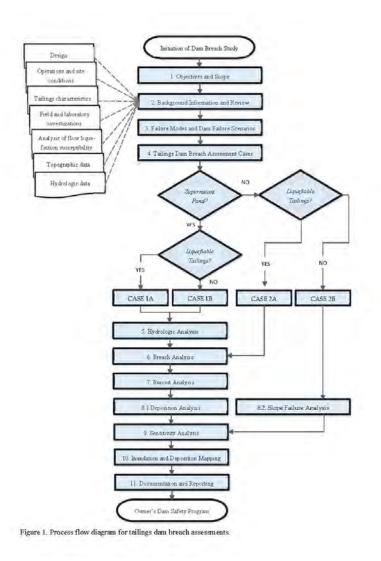
CDA (2019) Technical bulletin for tailings dam analyses provides the framework to assess the hazard due to tailings dam breach.

Hazard due to tailings breach is controlled by supernatant water that leads to fluidised flow with more impact. A breach flow without supernatant water will not fluidise resulting in non-fluid flow (Landslide and Debris Flow) with less impact.

The CDA (2019) process flow diagram for tailings dam breach assessment is in Plate 1.



#### PLATE 1 TAILINGS BREACH ASSESSMENT PROCESS FLOW DIAGRAM



Description of the Tailings Dam Breach Assessment Case is in CDA (2019) (Plate 2).



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#### PLATE 2 CDA (2019) TAILINGS DAM BREACH ASSESSMENT CASES

Presence of su- pernatant pond	Potential for tailings runout as a result of flow liquefaction*						
near the dam	Yes	No					
Yes	Case 1A – Liquefied Tailings with a Pond: Dam breach with flow of fluids and eroded and liquefied flowable tailings contributing additional volume of ma- terials released	Case 1B – Non-Liquefied tailings with a Pond: Dam breach with eroded tailings, transported and deposited by the flow of fluids					
No	Case 2A – Liquefied tailings without a Pond: Dam breach resulting from slope fail- ure with mudflow or debris type flow of liquefied flowable tailings (depend- ing on the degree of saturation)	Case 2B** – Non-Liquefied tailings without a Pond: Slope failure of the dam					

#### Table 1. Tailings dam breach assessment cases

\* Flow liquefaction of tailings could be induced by any potential trigger (static or cyclic/seismic) including shear strains in the tailings as a result of the dam breach (e.g., lateral unloading).

\*\* Hydrotechnical analyses or inundation mapping similar to other three cases would not be required for Case 2B. Landslide runout analysis may be more appropriate.

#### 9.6.2 ASSUMPTIONS

The PFMA in this assessment assumes:

- Surface water flows are controlled by perimeter drains with no potential inflow to the mine void;
- Capping progresses with mining;
- There is no potential for supernatant ponding; so that
- There is no credible Rainy Day failure scenario.



## 9.7 POTENTIAL FAILURE MODES

The PFMA for the Barrytown storage facility is in Appendix C.

The facility presents a Low Risk of failure primarily because:

- The location is free of significant geological hazard;
- There is no credible rainy day failure mode as the site is surrounded by diversion drains, the tailings surface is always < 3m below ground level and tailings are capped and finished to the completed surface with advance;
- There is no significant risk of earthquake or liquefaction induced instability;
- There is no containment bund with tailings always stored < 3m below ground surface;
- There is no potential for piping.

# 9.8 RESIDUAL RISK

Residual risk require that the facility:

- Closure criteria are confirmed early in the project life;
- An adequate knowledge base including suitable environmental modelling and mine performance is maintained;
- Storage plans are reviewed if chemicals are introduced into the system;
- Diversion drains are maintained;
- Trigger Action Response Plans are developed considering principal hazards prior to the start of mining;
- Emergency Plans are developed prior to the start of mining;
- Suitable governance is maintained over the site for the life of the project.

### 9.9 INSPECTION AND MONITORING

- Suitable inspection and monitoring at this stage is simply for walkover and standard mine record keeping.
- Environmental compliance monitoring which will likely be required should also be captured as part of the tailings monitoring plan.



# 10 REFERENCES

- International Council on Mining and Metals, United Nations Environment Programme, & Principles for Responsible Investment. (2020). Global Industry Standard on Tailings Management. In Global Tailings Review. <u>https://globaltailingsreview.org/wp-content/uploads/2020/08/global-industrystandard\_EN.pdf</u>
- Wells and Haverkamp (2020) Characterization of the Heavy Mineral Suite in a Holocene Beach Placer, Barrytown, New Zealand. Minerals 2020, 10, 86. <u>https://doi.org/10.3390/min10020086</u>.
- Look, B (2007) Handbook of geotechnical investigation and design tables. Taylor& Francis Group publ.

# 11 LIMITATIONS

- This report has been prepared for the particular purpose outlined in the project brief and no responsibility is accepted for the use of any part in other contexts or for any other purpose.
- No responsibility is accepted by Resource Development Consultants Ltd for inaccuracies in data supplied by others. Where data has been supplied by others, it has been assumed that this information is correct.
- This report is provided for sole use by the client and is confidential to the client and their professional advisors. No responsibility whatsoever for the contents of this report shall be accepted for any person other than the client.



# 12 CLOSURE

We trust this meets your current needs. Should you wish to discuss any aspect of the contents of this document please contact the undersigned on 06 877-1652.

Sincerely,

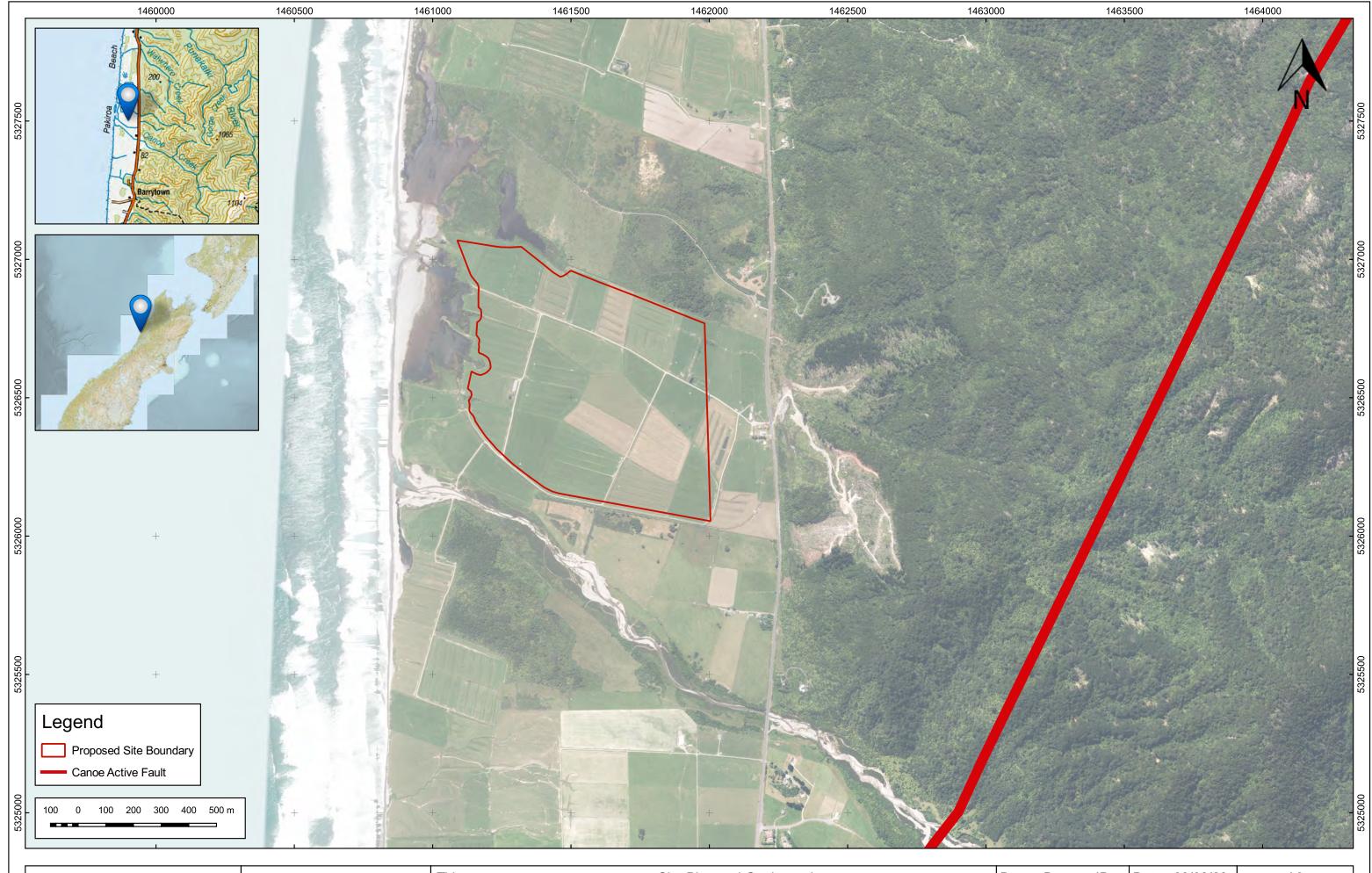
Prepared by:

Approved by:

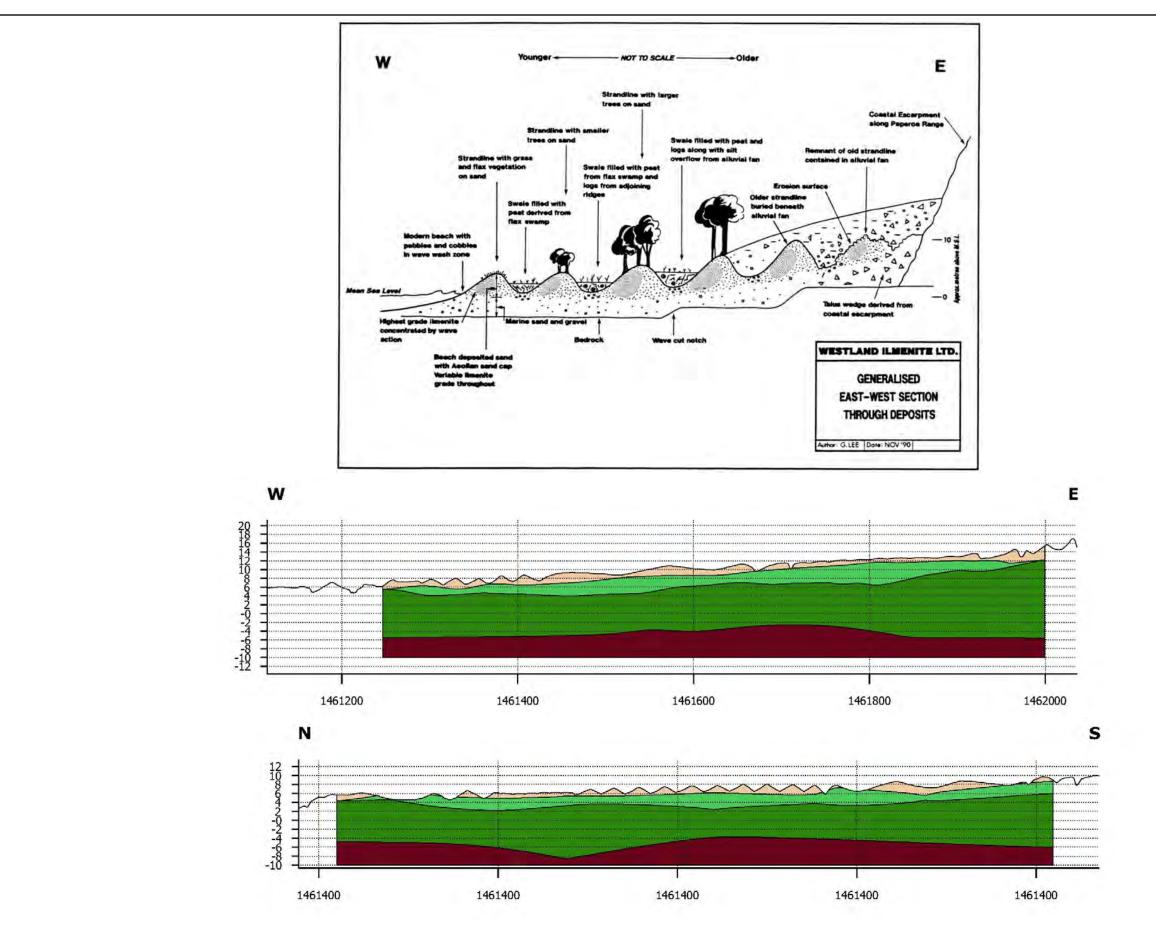


C A Wylie MSc; CMEngNZ; CPEng Principal





	RDCL	Title	Site Plan and Geohazards	Drawn By	ID	Date 08/02/23	A3
RDCL	PO Box 28057   8/308 Queen St East Hastings NZ Tel: +64 6 8771652   Fax: +64 6 877 5015	Project	Barrytown Mineral Sands Project	Checked By	CW	Date 10/02/23	
	Email: info@rdcl.co.nz www.rdcl.co.nz	Client	TiGa Minerals and Metals Ltd	Approved By		Date	Figure 1

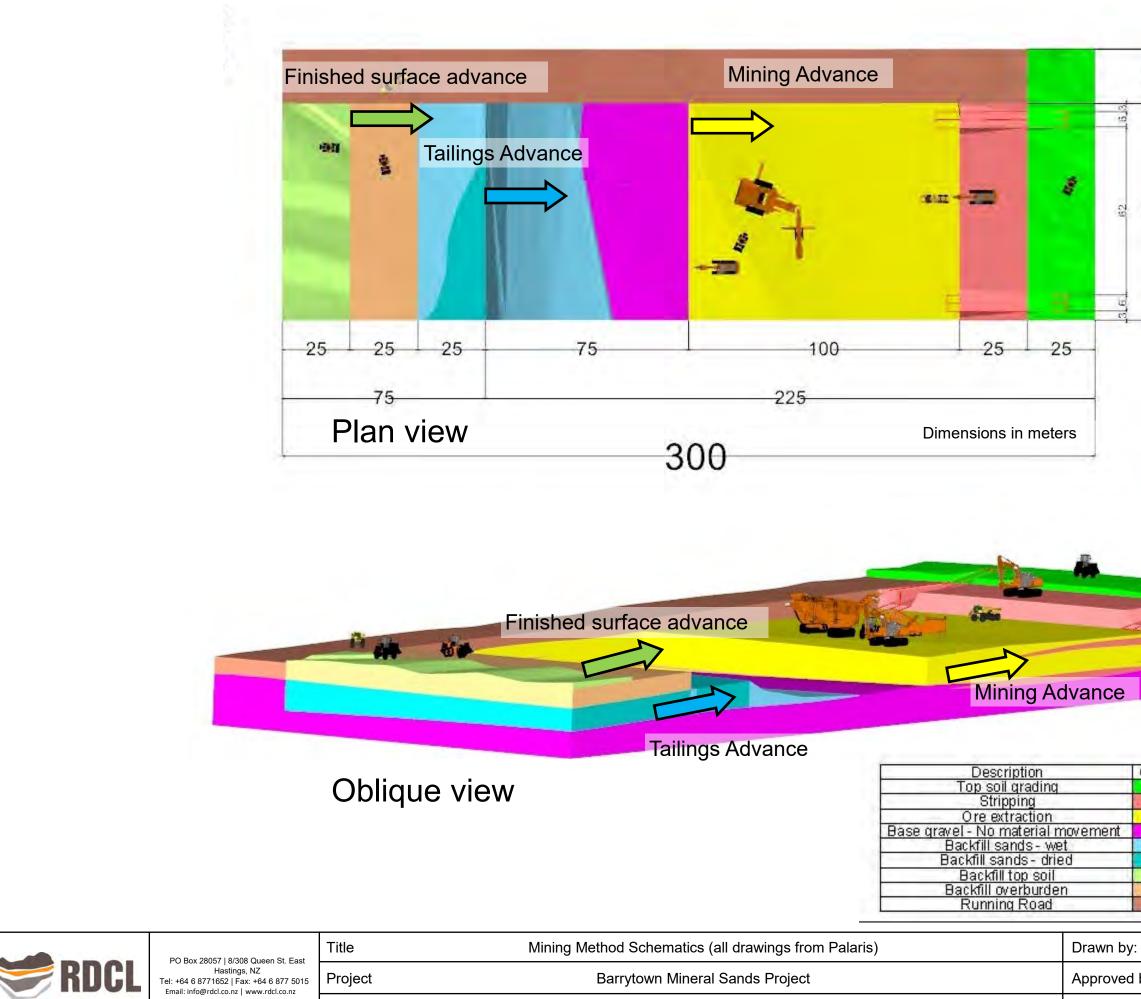


	PO Box 28057   8/308 Queen St. East	Title	Geological Setting and Schematic Cross-sections (from RSC Consultants)	Drawn by:	Date:	Drawing Size - A3
Hastings, NZ Tel: +64 6 8771652   Fax: +64 6 877 5015 Email: info@rdcl.co.nz   www.rdcl.co.nz		Project	Barrytown Mineral Sands Project	Approved by: CW	Date: 26/11/22	Figure Number
		Client	TiGa Minerals & Metals Ltd	RDCL Project No:	220986	02

# Legend

# **Barrytown GM Simple**

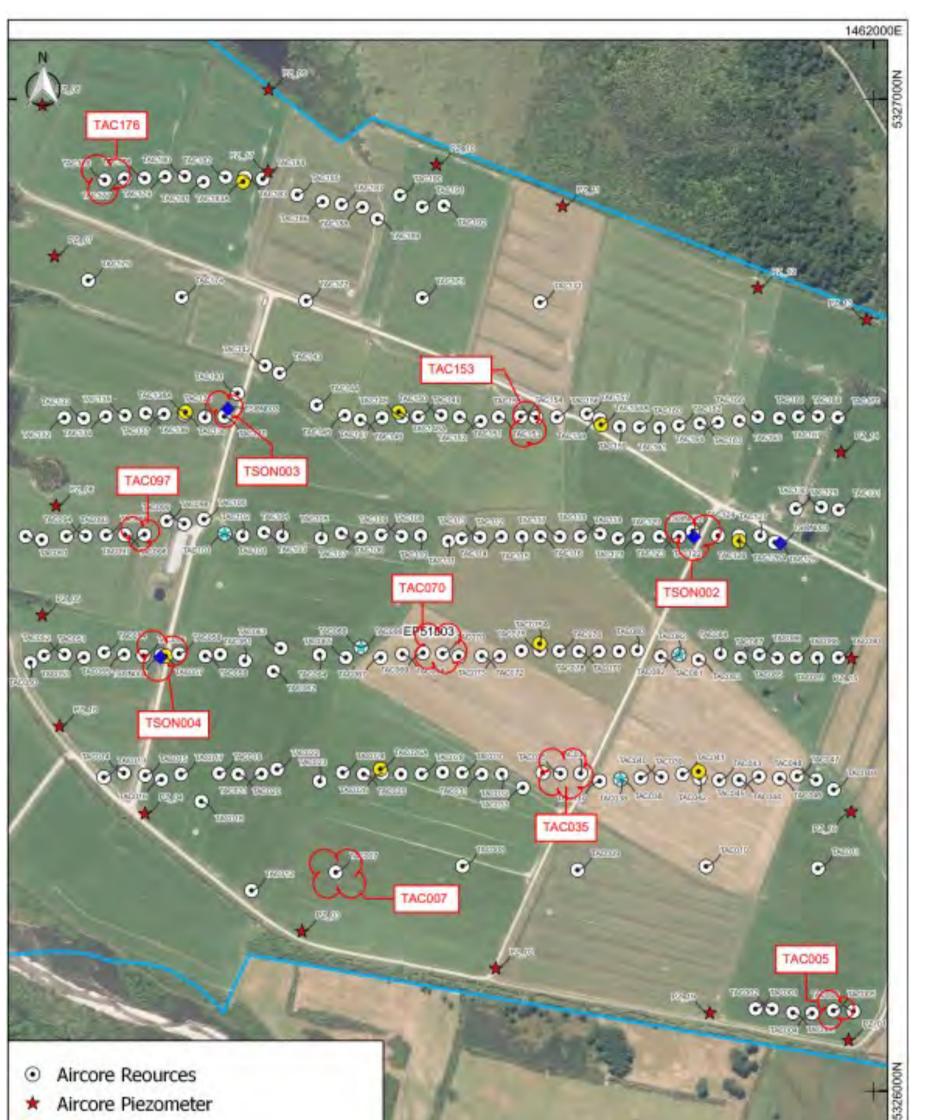
Sands	Top Clay-Silt-Gravels
Soil	Barren Gravel Base



Client TiGa Minerals & Metals Ltd

**RDCL** Proj

	Description Top Soil Overburden Sands Gravel Vvet Tail Overburden Backfille Top Soil Backfille	
EL	Date: 10/22	Drawing Size - A3
by: CW	Date: 26/11/22	Figure Number

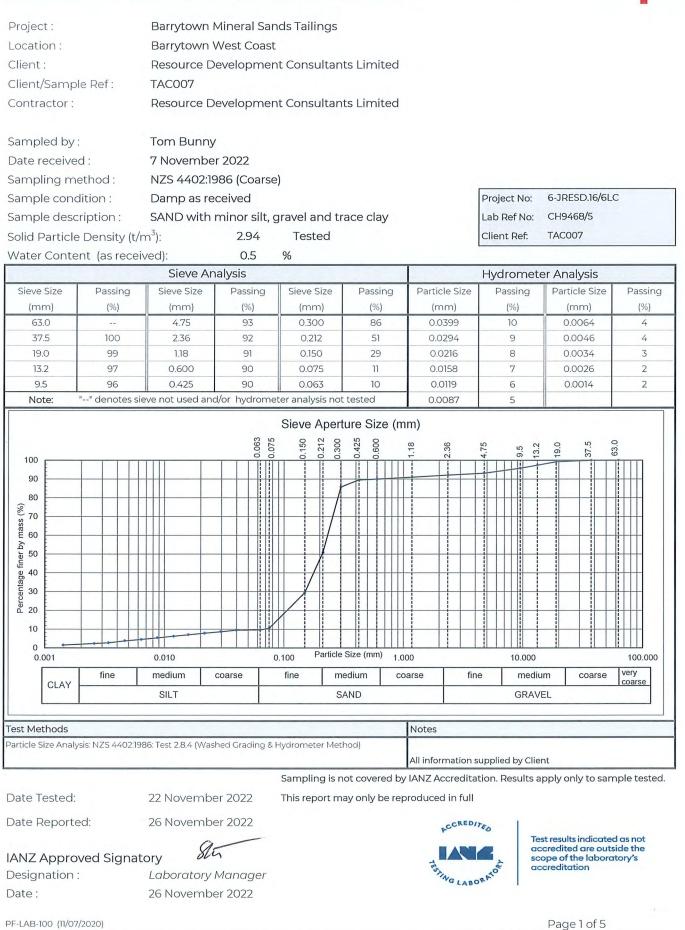


<ul><li>Mini P</li><li>Sonic</li></ul>	e Twin iezometer rce Consent Area (0	Coates Blo	ck)	*	• •/	the second	
14.July 22 CRS: NZGD2000 Data Source: LINZ			TISC INCOM		0	0.1 0.	2 km
		1				1	
	PO Box 28057   8/308 Queen St. East	Title	Exploration Samples	0	Drawn by:		Drawing Size-A
🥏 RDCL	Hastings, NZ Tel: +64 8771652 Fax: +64 6 877 5015 Email: info@rdcl.co.nz   www.rdcl.co.nz	Project	Barrytown Mineral Sands Pro	ject A	Approved by: CW	Date: 26/11/22	Figure Numb
		Client	TiGa Minerals & Metals Ltd	F	RDCL Project No:	220986	04

# APPENDIX A LABORATORY TEST RESULTS



# PARTICLE SIZE ANALYSIS (HYDROMETER METHOD) **TEST REPORT**



PF-LAB-100 (11/07/2020)

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Telephone +64 3 343 0739

Website www.wsp.com/nz

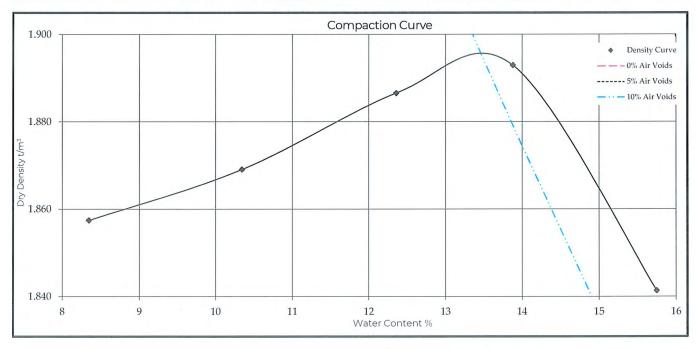
### DRY DENSITY / WATER CONTENT RELATIONSHIP STANDARD COMPACTION



6-JRESD.16/6LC CH9468/5 **TAC007** 

Project :	Barrytow	n Mineral Sands Tailii	ngs
Location :	Barrytow	n West Coast	
Client:	Resource	Development Consu	ultants Limited
Contractor :	Resource	Development Consu	Iltants Limited
Sampled by :	Tom Bun	ny	
Date sampled :	17 Octobe	er 2022	
Sampling method :	NZS 4402	2: 1986 (Coarse)	
Sample description :	Silty SAN	D witrh minor gravel	and trace clay
Sample condition :	Damp as	received	Project No :
Solid density :	2.94	t/m <sup>3</sup> (Tested)	Lab Ref No :
Source :	TAC007		Client Ref No :

	Contract The		Т	est Results	and the first				
Maximum dry de	ensity	1.89	t/m <sup>3</sup>		Natural wate	er content	0.5	%	
Optimum water content			%	Fraction tested Passin			Passing 19.0m	ing 19.0mm	
Sample ID		+8%	+10%	+12%	+14%	+16%	+18%		
Bulk density	t/m <sup>3</sup>	2.012	2.062	2.120	2.156	2.131	2.095		
Water content	%	8.3	10.3	12.4	13.9	15.8	17.3		
Dry density	t/m³	1.857	1.869	1.887	1.893	1.841	1.785		
Sample condition	า	Wet	Wet	Wet	Wet	Wet	Saturated		
		Firm	Firm	Firm	Firm	Soft	Soft		



Test Methods Notes NZS 4402 : 1986 Test 4.1.1 (Standard) Compaction All information supplied by Client

Date tested : 21 November 2022 Date reported : 26 November 2022 Sampling is not covered by IANZ Accreditation. Results apply only to sample tested. This report may only be reproduced in full

#### IANZ Approved Signatory

Sta

Date :

Designation : Laboratory Manager 26 November 2022



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Page 2 of 5

Telephone +64 3 343 0739 Website www.wsp.com/nz

Project :	Barrytown Mineral Sands Tai	lings	
Location :	<b>Barrytown West Coast</b>		
Client :	<b>Resource Development Consul</b>	tants Limited	
Contractor :	<b>Resource Development Consul</b>	tants Limited	
Sampled by :	Tom Bunny		
Date sampled :	17 October 2022		
Sampling method :	NZS 4402: 1986 (Coarse)		
Sample description :	SAND with minor silt, gravel a	and trace clay	
Sample condition :	Dry as received	<b>Project No :</b>	6-JRESD.16/6LC
Source:	<b>TAC007</b>	Lab Ref No :	CH9468/5
		<b>Client Ref No :</b>	<b>TAC007</b>

16

# **Test Results** Solid Density (t/m<sup>3</sup>): 2.94 Test Method: Determination of the Solid Density of Soil Particles NZS 4402 : 1986 : Test 2.7.1 (Passing 19.0mm)

Date tested : Date reported :

21 November 2022 26 November 2022

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Designation : Date :

Laboratory Manager 26 November 2022

PF-LAB-004 ( 30/05/2013)

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Telephone +64 3 343 0739 Facsimile Website www.wspopus.co.nz

#### PLASTICITY INDEX FOR AGGREGATES **TEST REPORT**

Project :	Barrytown Mineral Sands Tailing	S	
Location :	Barrytown West Coast		
Client :	Resource Development Consulta	ants Limited	
Contractor:	Resource Development Consulta	ants Limited	
Sampled by :	Tom Bunny		
Date sampled :	17 October 2022		
Sampling method :	NZS 4402: 1986 (Coarse)		
Sample description :	SAND with minor silt, gravel and	trace clay	
Sample condition :	As Received	Project No :	6-JRESD.16/6LC
Source :	TAC007	Lab Ref No :	CH9468/5
		Client Ref No :	TAC007

	Test Results	
Client Ref No :	TAC007	
Cone penetration limit :	23	
Plastic limit :	Unable to Roll Threads	
Plasticity index :	NP	
Sample fraction :	Fraction passing 425µm test sieve	
As received water content :	0.5	
Test Methods		
Water Content NZS	5 4407 : 2015 Test 3.1	

Water Content	NZS 4407 : 2015 Test 3.1	
Cone Penetration	NZS 4407 : 2015 : Test 3.2	
Plastic Limit	NZS 4407 : 2015 : Test 3.3	
Plasticity Index	NZS 4407 : 2015 : Test 3.4	

Date	tested :	22	November	2022
Date	reported :	26	November	2022

Sampling is not covered by IANZ Accreditation. Results apply only to sample tested. This report may only be reproduced in full All information supplied by Client

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PF-LAB-053 (09/06/2021)

Date :

Designation : Laboratory Manager 26 November 2022

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#### LINEAR SHRINKAGE TEST REPORT

Project :	Barrytown Mineral Sands Tailings
Location :	Barrytown West Coast
Client :	Resource Development Consultants Limited
Contractor :	Resource Development Consultants Limited
Sampled by :	Tom Bunny
Date sampled :	17 October 2022
Sampling method :	NZS 4402: 1986 (Coarse)
Sample description :	SAND with minor silt, gravel and trace clay
Sample condition :	Dry as received
Source:	TAC007



Project No :	6-JRESD.16/6LC	
Lab Ref No :	CH9468/5	
Client Ref No :	TAC007	

		Test R	esults	
Linear Shrinkage	e (%):	0		
0				
			las.	
Test Methods Linear Shrinka	ge NZS 4402 : 1986, Te	st 26	Notes Materials used: Pas	ssing 425um sieve
	ge 1123 4402 . 1500, re	502.0		Sing izsunsieve
Date tested :	22 November 2022	Sampling i	s not covered by IANZ A	ccreditation. Results apply only to sample teste
Date reported :	26 November 2022	This report	t may only be reproduced	d in full
IANZ Approv	red Signatory		PCCREDITED	
THIS APPLOV	Str			Test results indicated as not accredited are outside the
Designation :	Laboratory Manager		TEST TO	scope of the laboratory's accreditation
Date :	26 November 2022		STING LABORATO	
PF-LAB-101 (30/05/20	013)			Page 5 of 5

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	TEST R	REPORT						
Project :	Barrytowr	n Mineral Sar	nds Tailings					
ocation :		n West Coast						
client :		Developmer		ts l imited				
ilient/Sample Ref :		Developmen	it consultan	LI III III CU				
Contractor :		Developmer	t Concultan	telimitod				
Untractor.	Resource	Developmen	it Consultan	IS LIMITED				
ampled by :	Tom Bunr	ıy						
ate received :	7 Novemb	er 2022						
ampling method	: NZS 4402	1986 (Coarse	e)					
ample condition :	Damp as r	received				Project No:	6-JRESD.16/6L	С
ample descriptior	n : Silty SANE	) with trace g	gravel and cl	ау		Lab Ref No:	CH9468/6	
olid Particle Dens	ity (t/m <sup>3</sup> ):	2.68	Tested			Client Ref:	TAC035	
Vater Content (as	received):	0.2	%					
	Sieve A	nalysis				Hydromete	r Analysis	
Sieve Size Pas	sing Sieve Size	Passing	Sieve Size	Passing	Particle Size	Passing	Particle Size	Passing
(mm) (9		(%)	(mm)	(%)	(mm)	(%)	(mm)	(%)
	- 4.75	97	0.300	87	0.0431	28	0.0069	12
37.5 - 19.0 10		95	0.212	72 47	0.0316	26 24	0.0051	8
13.2 9		92	0.075	34	0.0170	21	0.0028	5
9.5 9	9 0.425	90	0.063	32	0.0128	19	0.0015	4
Note: "" den	otes sieve not used a	nd/or hydrome	ter analysis not	tested	0.0095	15		
100 90 80 (%) ssee 60 40 50 10 0 0.001	0.010			Size (mm) 1.0	2.36	10.000	19.0	
fine		coarse			arse fin		n coarse	very
CLAY	SILT		S/	AND		GRAVE		coarse
st Methods					Notes			
rticle Size Analysis: NZS 4	4402:1986: Test 2.8.4 (W	ashed Grading & I	Hydrometer Meth	nod)				
					All information s	upplied by Clien	t	
ate Tested:	23 Noveml	oer 2022			IANZ Accredita	tion. Results ap	oply only to san	nple tested
				5				
ate Reported: ANZ Approved Si	29 Noveml	oer 2022			PCCRED/		t results indicated credited are outs pe of the labora creditation	side the

IANZ Approved Signatory Designation : Date :

PF-LAB-100 (11/07/2020)

Laboratory Manager 29 November 2022



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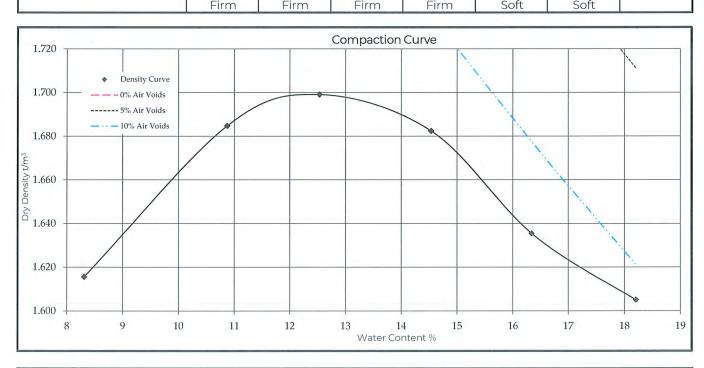
Pagel of 4

WSP Christchurch (Hayton Rd) Quality Management Systems Certified to ISO 9001

52C Hayton Rd PO Box 1482, Christchurch Mail Centre, 8140, Christchurch, New Zealand

Project :	Barrytov	vn Mineral Sands Taili	ngs			
Location :	Barrytown West Coast					
Client :	Resource	e Development Consi	ultants Limited			
Contractor :	Resource Development Consultants Limited					
Sampled by :	Tom Bunny					
Date sampled :	17 October 2022					
Sampling method :	NZS 4402: 1986 (Coarse)					
Sample description :	Silty SAN	ID with trace gravel a	nd clay			
Sample condition :	Damp as	s received	Project No :	6-JRESD.16/6LC		
Solid density :	2.68	t/m <sup>3</sup> (Tested)	Lab Ref No :	CH9468/6		
Source :	TAC035 Client Ref No : TAC035					

		1312	Т	est Results		and the second		1
Maximum dry de	ensity	1.70	t/m³		Natural wat	er content	0.2	%
Optimum water content		12.0 %			Fraction tested Passing 19.0m		ım	
Sample ID		+8%	+10%	+12%	+14%	+16%	+18%	
Bulk density	t/m <sup>3</sup>	1.750	1.868	1.912	1.927	1.903	1.897	
Water content	%	8.3	10.9	12.5	14.5	16.3	18.2	
Dry density	t/m <sup>3</sup>	1.616	1.685	1.699	1.682	1.635	1.605	
Sample condition	n	Wet	Wet	Wet	Wet	Wet	Saturated	
		Linne	Firms	Firm	Firms	Coft	Coft	



Test Methods Notes NZS 4402 : 1986 Test 4.1.1 (Standard) Compaction All information supplied by Client

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8140, Christchurch, New Zealand

Date tested : 22 November 2022 Date reported : 29 November 2022 Sampling is not covered by IANZ Accreditation. Results apply only to sample tested. This report may only be reproduced in full

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Date :

Designation: Laboratory Manager 29 November 2022

PF-LAB-026 (10/07/20)

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Page 2 of 4

TE	CST REPORT		
Project :	Barrytown Mineral Sands Tail	ings	
Location :	<b>Barrytown West Coast</b>		
Client :	<b>Resource Development Consul</b>	tants Limited	
Contractor :	<b>Resource Development Consul</b>	tants Limited	
Sampled by :	Tom Bunny		
Date sampled :	17 October 2022		
Sampling method :	NZS 4402: 1986 (Coarse)		
Sample description :	Silty SAND with trace gravel a	nd clay	
Sample condition :	Dry as received	<b>Project No :</b>	6-JRESD.16/6LC
Source:	TAC035	Lab Ref No :	CH9468/6
		<b>Client Ref No:</b>	<b>TAC035</b>

Test	Results	
Solid Density (t/m <sup>3</sup> ):	2.68	
Test Method: Determination of the Solid Den (Passing 19.0mm)	ity of Soil Particles NZS 4402 : 1986 : 7	Test 2.7.1

Date tested :

23 November 2022 Date reported : 29 November 2022

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Laboratory Manager 29 November 2022

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Telephone +64 3 343 0739 Facsimile Website www.wspopus.co.nz

Page 3 of 4

#### PLASTICITY INDEX FOR AGGREGATES **TEST REPORT**

Project :	Barrytown Mineral Sands Tailings	
Location :	Barrytown West Coast	
Client:	Resource Development Consultants Li	mited
Contractor :	Resource Development Consultants Li	mited
Sampled by :	Tom Bunny	
Date sampled :	17 October 2022	
Sampling method :	NZS 4402: 1986 (Coarse)	
Sample description :	Silty SAND with trace gravel and clay	
Sample condition :	As Received	Project No :
Source :	TAC035	Lab Ref No :
		Client Ref No:

TAC035	
30	
Unable to Roll Threads	
NP	
Fraction passing 425µm test sieve	
0.2	
	30 Unable to Roll Threads NP Fraction passing 425µm test sieve

Test Methods		
Water Content	NZS 4407 : 2015 Test 3.1	
Cone Penetration	NZS 4407 : 2015 : Test 3.2	
Plastic Limit	NZS 4407 : 2015 : Test 3.3	
Plasticity Index	NZS 4407 : 2015 : Test 3.4	

Date tested : 22 November 2022 Date reported : 29 November 2022 Sampling is not covered by IANZ Accreditation. Results apply only to sample tested. This report may only be reproduced in full All information supplied by Client

IANZ Approved Signatory

Designation : Laboratory Manager Date :

PF-LAB-053 (09/06/2021)

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Telephone +64 3 343 0739 Website www.wsp.com/nz

6-JRESD.16/6LC CH9468/6

**TAC035** 

						_			
Project :		Barrytown I	Mineral Sar	nds Tailings					
Location :		Barrytown	West Coast						
Client :		Resource D	evelopmer	nt Consultar	nts Limited				
Client/Sample	e Ref:	TAC070							
Contractor :		Resource D	evelopmen	it Consultar	nts Limited				
Sampled by :		Tom Bunny							
Date received	1:	7 Novembe	r 2022						
Sampling me	thod :	NZS 4402:19	986 (Coarse	)					
Sample condi	tion :	Damp as re	ceived				Project No:	6-JRESD.16/6L	С
Sample descr	iption :	Silty SAND	with trace g	gravel and c	lay		Lab Ref No:	CH9468/7	
Solid Particle	Density (t/r	m <sup>3</sup> ):	3.00	Tested			Client Ref:	TAC070	
Water Conter	nt (as receiv	ved):	0.1	%					
		Sieve An	alysis				Hydromete	er Analysis	
Sieve Size	Passing	Sieve Size	Passing	Sieve Size	Passing	Particle Size	Passing	Particle Size	Passing
(mm)	(%)	(mm)	(%)	(mm)	(%)	(mm)	(%)	(mm)	(%)
63.0		4.75	99	0.300	96	0.0381	22	0.0063	9
37.5		2.36	99	0.212	81	0.0281	20	0.0047	6
19.0	100	1.18 0.600	98	0.150	53 26	0.0209	18	0.0034	4 3
9.5	100	0.425	98	0.073	20	0.0133	13	0.0020	3
	" denotes sie	eve not used and	d/or hydrome	11		0.0087	11		
100 90 80 (%) 70 see 60 Aq asy 20 10 0.001 CLAY	fine	0.010 medium	00000000000000000000000000000000000000	fine m	Size (mm) 1.00	00         00           arse         fir	Li         G         N           I         I         I	m coarse	100.000
Test Methods						Notes			
Particle Size Analysi	s: NZS 4402:198	36: Test 2.8.4 (Was	hed Grading & H	Hydrometer Met	hod)	All information :	supplied by Clier	nt	
Date Tested:		23 Novembe	er 2022			IANZ Accredita	ition. Results a	pply only to san	ple tested.
	4.								
Date Reported	ed Signato	29 November ory <i>Ji</i> Laboratory I				FEITING LABO		st results indicated credited are outs ope of the labora creditation	ide the

PF-LAB-100 (11/07/2020)

Date :

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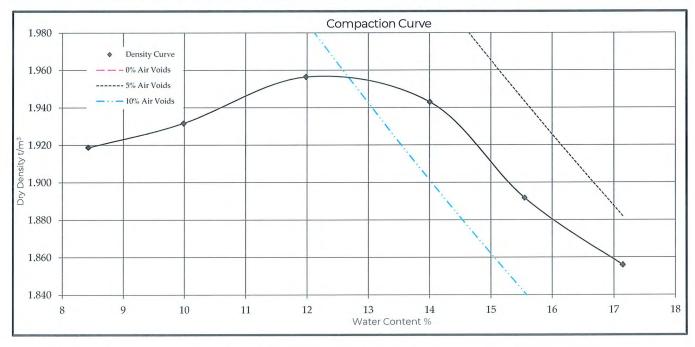
29 November 2022

52C Hayton Rd PO Box 1482, Christchurch Mail Centre, 8140, Christchurch, New Zealand Page 1 of 4

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Project :	Barrytow	n Mineral Sands Taili	ngs	
Location :	Barrytow	n West Coast		
Client :	Resource	e Development Consu	ultants Limited	
Contractor :	Resource	e Development Consu	ultants Limited	
Sampled by :	Tom Bur	nny		
Date sampled :	17 Octob	er 2022		
Sampling method :	NZS 440	2: 1986 (Coarse)		
Sample description :	Silty SAN	ID with trace gravel a	nd clay	
Sample condition :	Damp as	received	Project No :	6-JRESD.16/6LC
Solid density :	3.00	t/m³ (Tested)	Lab Ref No :	CH9468/7
Source :	TAC070		Client Ref No :	TAC070

			Т	est Results				
Maximum dry density		1.96	t/m <sup>3</sup>		Natural wat	er content	0.1	%
Optimum water content 12.0			%		Fraction tes	ted	Passing 19.0m	nm
Sample ID	-	+8%	+10%	+12%	+14%	+16%	+18%	1
Bulk density	t/m <sup>3</sup>	2.081	2.125	2.191	2.215	2.186	2.174	
Water content	%	8.4	10.0	12.0	14.0	15.6	17.1	
Dry density	t/m <sup>3</sup>	1.919	1.932	1.956	1.943	1.892	1.856	
Sample condition	า	Wet	Wet	Wet	Wet	Wet	Saturated	
		Firm	Firm	Firm	Firm	Soft	Soft	



Test Methods Notes NZS 4402 : 1986 Test 4.1.1 (Standard) All information supplied by Client Compaction

52C Hayton Rd

PO Box 1482, Christchurch Mail Centre,

8140, Christchurch, New Zealand

Date tested : 21 November 2022 Date reported : 29 November 2022 Sampling is not covered by IANZ Accreditation. Results apply only to sample tested. This report may only be reproduced in full

IANZ Approved Signatory

Date :

Sty Designation: Laboratory Manager 29 November 2022

PF-LAB-026 (10/07/20)

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Project :	Barrytown Mineral Sands Ta	ilings	
Location :	Barrytown West Coast	inings	
Client :	Resource Development Const	Iltants Limited	
Contractor :	Resource Development Const	ltants Limited	
Sampled by :	Tom Bunny		
Date sampled :	17 October 2022		
Sampling method :	NZS 4402: 1986 (Coarse)		
Sample description :	Silty SAND with trace gravel	and clay	
Sample condition :	Dry as received	Project No :	6-JRESD.16/6LC
Source:	<b>TAC070</b>	Lab Ref No :	CH9468/7
		<b>Client Ref No :</b>	<b>TAC070</b>

	Test Results
	Solid Density (t/m <sup>3</sup> ): 3.00
	Test Method: Determination of the Solid Density of Soil Particles NZS 4402 : 1986 : Test 2.7.1 (Passing 19.0mm)
Date tested : Date reported :	24 November 202229 November 2022This report may only be reproduced in full

Approved

Sty

Designation : Date :

Laboratory Manager 29 November 2022

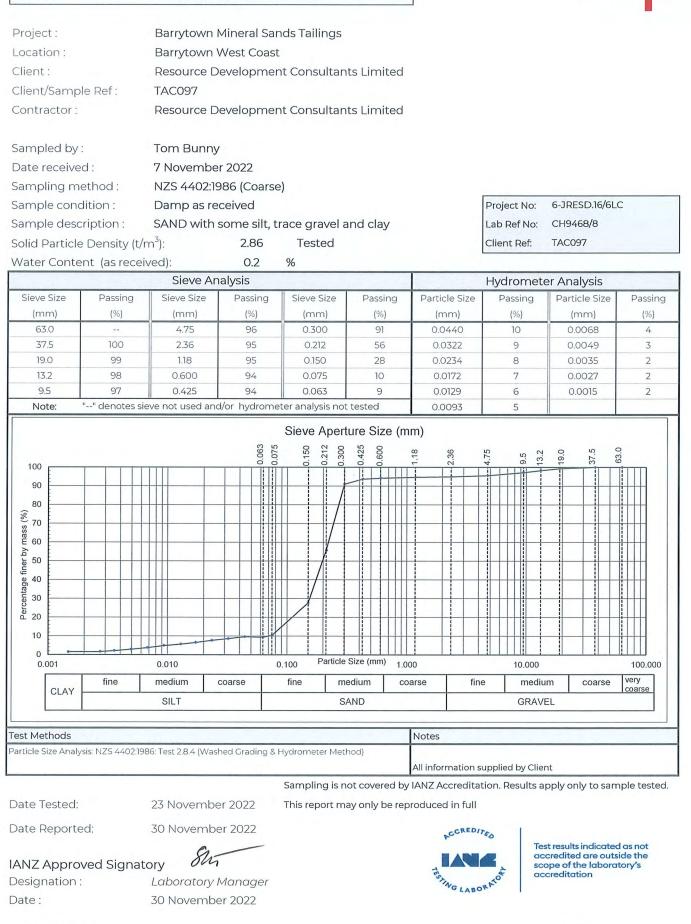
PF-LAB-004 ( 30/05/2013)

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PF-LAB-100 (11/07/2020)

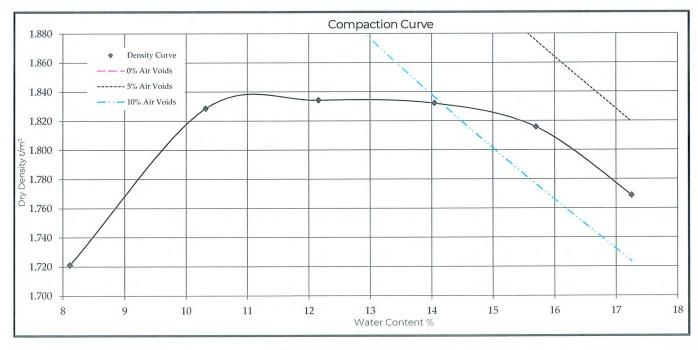
52C Hayton Rd PO Box 1482, Christchurch Mail Centre, 8140, Christchurch, New Zealand Page 1 of 4

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6-JRESD.16/6LC CH9468/8 **TAC097** 

Project :	Barrytown Mineral Sand	s Tailings
Location :	Barrytown West Coast	
Client :	Resource Development	Consultants Limited
Contractor :	Resource Development	Consultants Limited
Sampled by :	Tom Bunny	
Date sampled :	17 October 2022	
Sampling method :	NZS 4402: 1986 (Coarse)	
Sample description :	SAND with some silt, tra	ce gravel and clay
Sample condition :	Damp as received	Project No :
Solid density :	2.86 t/m <sup>3</sup> (Tested	) Lab Ref No :
Source :	TAC097	Client Ref No :

			Т	est Results				_
Maximum dry density		1.84	t/m³		Natural wat	er content	0.2	%
Optimum water content 11.0			%		Fraction tes	ted F	Passing 19.0mr	n
Sample ID		+8%	+10%	+12%	+14%	+16%	+18%	
Bulk density	t/m <sup>3</sup>	1.861	2.017	2.057	2.089	2.101	2.074	
Water content	%	8.1	10.3	12.2	14.0	15.7	17.3	
Dry density	t/m <sup>3</sup>	1.721	1.829	1.834	1.832	1.816	1.769	
Sample condition	า	Wet	Wet	Wet	Wet	Wet	Saturated	
		Firm	Firm	Firm	Firm	Soft	Soft	



Test Methods Notes NZS 4402 : 1986 Test 4.1.1 (Standard) All information supplied by Client Compaction

52C Hayton Rd

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8140, Christchurch, New Zealand

Date tested : 29 November 2022 Date reported : 30 November 2022 Sampling is not covered by IANZ Accreditation. Results apply only to sample tested. This report may only be reproduced in full

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SG

Date :

Designation : Laboratory Manager 30 November 2022

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Page 2 of 4

Project :	Barrytown Mineral Sands Ta	ailings		
Location :	<b>Barrytown West Coast</b>			
Client :	<b>Resource Development Cons</b>	ultants Limited		
Contractor :	<b>Resource Development Cons</b>	ultants Limited		
Sampled by :	Tom Bunny			
Date sampled :	17 October 2022			
Sampling method :	NZS 4402: 1986 (Coarse)			
Sample description :	SAND with some silt, trace g	ravel and clay		
Sample condition :	Dry as received	<b>Project No :</b>	6-JRESD.16/6LC	
Source:	<b>TAC097</b>	Lab Ref No :	CH9468/8	
		<b>Client Ref No :</b>	<b>TAC097</b>	
		-		

#### **Test Results**

Solid Density (t/m<sup>3</sup>):

2.86

Test Method: Determination of the Solid Density of Soil Particles NZS 4402 : 1986 : Test 2.7.1 (Passing 19.0mm)

Date tested : 24 Date reported : 30

24 November 2022 30 November 2022

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815

Designation : Date : Laboratory Manager 30 November 2022

PF-LAB-004 ( 30/05/2013)

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#### PLASTICITY INDEX FOR AGGREGATES **TEST REPORT**

Project :	Barrytown Mineral Sands Tail	ings					
Location :	Barrytown West Coast						
Client :	Resource Development Cons	ultants Limited					
Contractor:	Resource Development Cons	Resource Development Consultants Limited					
Sampled by :	Tom Bunny						
Date sampled :	17 October 2022						
Sampling method :	NZS 4402: 1986 (Coarse)						
Sample description :	SAND with some silt, trace gr	avel and clay					
Sample condition :	As Received	Project No :	6-JRESD.16/6LC				
Source :	TAC097	Lab Ref No :	CH9468/8				
		Client Ref No :	TAC097				

	Test Results	
Client Ref No :	TAC097	
Cone penetration limit :	26	
Plastic limit :	Unable to Roll Threads	
Plasticity index :	NP	
Sample fraction :	Fraction passing 425µm test sieve	
As received water content :	0.2	
Test Methods		
Water Content	NZS 4407 : 2015 Test 3.1	
Cone Penetration	NZS 4407 : 2015 : Test 3.2	

Date tested :	25 November 2022
Date reported :	30 November 2022

IANZ Approved Signatory 815

NZS 4407 : 2015 : Test 3.3

NZS 4407 : 2015 : Test 3.4

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Designation : Laboratory Manager 30 November 2022

PF-LAB-053 (09/06/2021)

Date :

Plastic Limit

Plasticity Index

WSP Christchurch (Hayton Rd) Quality Management Systems Certified to ISO 9001

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Project : Location : Client : Client/Sam Contractor Sampled & Date recei Sampling Sample co Sample de Solid Parti	r : ved : method : ndition :		West Coast Developmer Developmer 2 er 2022 D86 (Coarse D86 (Coarse	nt Consultar nt Consultar	nts Limited	slay	Project No: Lab Ref No: Client Ref:	6-JRESD.16/6L CH9468/9 TAC153	C
	itent (as recei		0.2	%					
		Sieve Ar	nalysis				Hydromete	er Analysis	
Sieve Size	Passing	Sieve Size	Passing	Sieve Size	Passing	Particle Size	Passing	Particle Size	Passing
(mm)	(%)	(mm)	(%)	(mm)	(%)	(mm)	(%)	(mm)	(%)
63.0		4.75	91	0.300	80	0.0394	11	0.0061	6
37.5	100	2.36	87 84	0.212	64 36	0.0286	10	0.0044	5
13.2	98	0.600	83	0.150	13	0.0209	8	0.0032	3
9.5	95	0.425	82	0.063	12	0.0115	7	0.0013	2
Note:	"" denotes si	eve not used and	d/or hydrome	ter analysis no	ot tested	0.0083	7		
100 90 80 (%) sse 60 40 90 90 80 (%) sse 60 40 90 90 90 90 90 90 90 90 90 90 90 90 90	Y fine	0.010 medium SILT	90000000000000000000000000000000000000	09100 Particl	e Size (mm) 1.0	82         82           1         1	9 7 8 7 8 7 8 7 8 7 8 7 8 7 8 7 8 7 8 7	m coarse	22 1 1 1 1 1 1 1 1 1 1 1 1 1
	alysis: NZS 4402:19	86: Test 2.8.4 (Was	hed Grading & I	Hydrometer Me	thod)	Notes			
Date Teste		29 Novembe		Sampling is	not covered by	All information s ANZ Accredita			nple tested.
Date Repo	rted:	30 Novemb	er 2022		,,	PCCRED/	Tes	st results indicated	
IANZ App Designatio Date :	roved Signat n :	Laboratory 30 Novemb	Manager			TESTING LABO		ope of the labora creditation	

PF-LAB-100 (11/07/2020)

52C Hayton Rd PO Box 1482, Christchurch Mail Centre, 8140, Christchurch, New Zealand Page 1 of 4

115

Barrytov	vn Mineral Sands Taili	ngs	
Barrytow	vn West Coast		
Resource	e Development Cons	ultants Limited	
Resource	e Development Cons	ultants Limited	
Tom Bur	nny		
17 Octob	er 2022		
NZS 440	2: 1986 (Coarse)		
SAND wi	ith some gravel, mind	or silt and trace clay	
Damp as	s received	Project No :	6-JRESD.16/6LC
3.16	t/m <sup>3</sup> (Tested)	Lab Ref No :	CH9468/9
<b>TAC153</b>		Client Ref No :	TAC153
	Barrytov Resource Tom Bur 17 Octob NZS 440 SAND w Damp as 3.16	Barrytown West Coast Resource Development Const Resource Development Const Tom Bunny 17 October 2022 NZS 4402: 1986 (Coarse) SAND with some gravel, mino Damp as received 3.16 t/m <sup>3</sup> (Tested)	Resource Development Consultants LimitedResource Development Consultants LimitedTom Bunny17 October 2022NZS 4402: 1986 (Coarse)SAND with some gravel, minor silt and trace clayDamp as received3.16t/m³ (Tested)

			Т	est Results				
Maximum dry de	ensity	2.24	t/m³		Natural wa	ter content	0.2	%
Optimum water content		8.0	%	Fraction tested Passing 19.0r				
Sample ID		+4%	+6%	+8%	+10%	+12%		
Bulk density	t/m³	2.114	2.226	2.425	2.413	2.320		
Water content	%	4.1	6.0	8.0	9.7	11.7		
Dry density	t/m³	2.031	2.100	2.244	2.199	2.078		
Sample condition		Moist	Wet	Wet	Wet	Saturated		
		Firm	Firm	Firm	Soft	Soft		

2.260				Compa	action Curve			
2.240							` + [	ensity Curve
2.220								% Air Voids
2.200						1		% Air Voids
2.180								
			/					
2.160 2.140 2.120							in the second se	
2 120								
2 100								
2.100 2.080		/	1					
2.060								•
2.040								
2.040	*							
2.000								
4	1	5	6		-	9 1	0	11
				Wa	ater Content %			

Test Methods Notes NZS 4402 : 1986 Test 4.1.1 (Standard) All information supplied by Client Compaction

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PO Box 1482, Christchurch Mail Centre,

8140, Christchurch, New Zealand

Date tested : 24 November 2022 Date reported : 30 November 2022

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#### IANZ Approved Signatory

Sta

Date :

Designation : Laboratory Manager 30 November 2022

PF-LAB-026 (10/07/20)

WSP

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Test results indicated as not accredited are outside the scope of the laboratory's accreditation

Page 2 of 4

Project :	Barrytown Mineral Sands Ta	ailings	
Location :	<b>Barrytown West Coast</b>		
Client :	<b>Resource Development Cons</b>	ultants Limited	
Contractor :	<b>Resource Development Cons</b>	ultants Limited	
Sampled by :	Tom Bunny		
Date sampled :	17 October 2022		
Sampling method :	NZS 4402: 1986 (Coarse)		
Sample description :	SAND with some gravel, min	or silt and trace clay	
Sample condition :	Dry as received	Project No :	6-JRESD.16/6LC
Source:	TAC153	Lab Ref No :	CH9468/9
		<b>Client Ref No :</b>	<b>TAC153</b>

# **Test Results** Solid Density (t/m<sup>3</sup>): 3.16 Test Method: Determination of the Solid Density of Soil Particles NZS 4402 : 1986 : Test 2.7.1 (Passing 19.0mm)

Date tested : Date reported :

28 November 2022 30 November 2022

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Approved

Str.

Laboratory Manager

Designation : Date :

30 November 2022

PF-LAB-004 ( 30/05/2013)

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Page 3 of 4

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#### PLASTICITY INDEX FOR AGGREGATES TEST REPORT

Project :	Barrytown Mineral Sands Tailings						
Location :	Barrytown West Coast						
Client :	Resource Development Consultar	its Limited					
Contractor :	Resource Development Consultar	Resource Development Consultants Limited					
Sampled by :	Tom Bunny						
Date sampled :	17 October 2022						
Sampling method :	NZS 4402: 1986 (Coarse)						
Sample description :	SAND with some gravel, minor silt	and trace clay					
Sample condition :	As Received	Project No :	6-JRESD.16/6LC				
Source :	TAC153	Lab Ref No :	CH9468/9				
		Client Ref No :	TAC153				

	Test Results	
Client Ref No :	TAC153	
Cone penetration limit :	18	
Plastic limit :	Unable to Roll Threads	
Plasticity index :	NP	
Sample fraction :	Fraction passing 425µm test sieve	
As received water content :	0.2	
Fest Methods		
Water Content	NZS 4407 : 2015 Test 3.1	
Cone Penetration	NZS 4407 : 2015 : Test 3.2	
Diactic Lipsit		

Cone PenetrationNZS 4407 : 2015 : Test 3.2Plastic LimitNZS 4407 : 2015 : Test 3.3Plasticity IndexNZS 4407 : 2015 : Test 3.4

Date tested : 30 November 2022 Date reported : 1 December 2022

IANZ Approved Signatory

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PF-LAB-053 (09/06/2021)

Date :

WSP Christchurch (Hayton Rd) Quality Management Systems Certified to ISO 9001

1 December 2022

52C Hayton Rd PO Box 1482, Christchurch Mail Centre, 8140, Christchurch, New Zealand

Project :	Barrytown N	Aineral San	ds Tailings					
ocation :	Barrytown V	Barrytown West Coast						
Client :	Resource De	evelopmen	t Consultan	ts Limited				
Client/Sample Ref :	TAC170							
Contractor :	Resource De	evelopmen	t Consultan	ts Limited				
ampled by :	Tom Bunny							
Date received :	7 November	2022						
ampling method :	NZS 4402:19	86 (Coarse	)					
ample condition :	Damp as rec		/			Project No:	6-JRESD.16/6L	С
ample description :	Gravelly SAN		me silt and t	race clav		Lab Ref No:	CH9468/10	
olid Particle Density		2.82	Tested	ruce city		Client Ref.	TAC170	
		0.3				Client Rei.	IACITO	
/ater Content (as re			%			1 bushes as a b	. A secharia	
Ciaux Ciae Dessia	Sieve Ana		Ciaux Ciau	Dessien	Dartiala Cina	Hydromete	1	Dessing
Sieve Size Passing (mm) (%)	g Sieve Size (mm)	Passing (%)	Sieve Size (mm)	Passing (%)	Particle Size (mm)	Passing (%)	Particle Size (mm)	Passing (%)
(mm) (%) 63.0	4.75	(%)	0.300	(%)	0.0421	(%)	0.0066	(%)
37.5 100	2.36	68	0.212	44	0.0309	18	0.0048	7
19.0 99	1.18	62	0.150	33	0.0226	16	0.0035	5
13.2 96	0.600	58	0.075	22	0.0166	14	0.0027	5
9.5 91	0.425	56	0.063	21	0.0124	13	0.0015	3
Note: "" denote	s sieve not used and,	/or hydromet	ter analysis not	t tested	0.0091	11		
90 80 80 80 80 70 80 70 60 40 50 90 80 90 80 90 80 90 80 90 80 90 90 80 90 90 90 90 90 90 90 90 90 9	0.010 medium c SILT	0 0 0	fine m	Size (mm) 1.00 edium coa	arse fin	10.000 me mediu GRAVI		100.000
est Methods					Notes			
rticle Size Analysis: NZS 440	2:1986: Test 2.8.4 (Wash	ned Grading & H	Hydrometer Met	hod)				
					All information s	supplied by Clier	nt	
			Sampling is n	ot covered by	IANZ Accredita	tion. Results a	pply only to san	nple tested
ate Tested:	29 Novembe	r 2022	This report m	ay only be rep	roduced in full			
ate Reported:	30 Novembe	r 2022						
ANZ Approved Sigr	hatory 80 Laboratory N	Manager			FUTING LABO		st results indicate credited are outs ope of the labora creditation	ide the
ate :	30 Novembe	er 2022						
							Page 1 of 4	

PF-LAB-100 (11/07/2020)

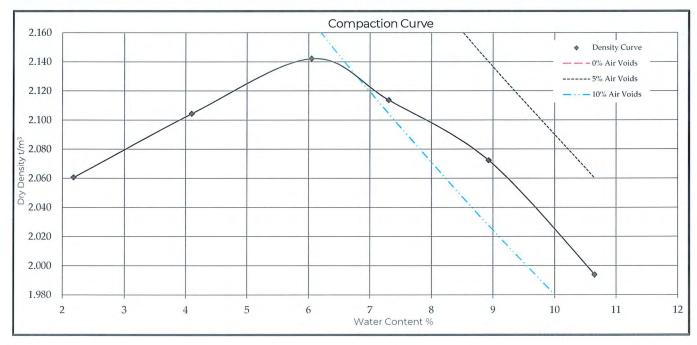
52C Hayton Rd PO Box 1482, Christchurch Mail Centre, 8140, Christchurch, New Zealand Telephone +64 3 343 0739

Website www.wsp.com/nz

**\\S**D

Project :	Barrytov	vn Mineral Sands Taili	ngs				
Location :	Barrytown West Coast						
Client :	Resource Development Consultants Limited						
Contractor :	Resource Development Consultants Limited						
Sampled by :	Tom Bunny						
Date sampled :	17 October 2022						
Sampling method :	NZS 4402: 1986 (Coarse)						
Sample description :	Gravelly	SAND with some silt a	and trace clay				
Sample condition :	Damp as	s received	Project No :	6-JRESD.16/6LC			
Solid density :	2.82	t/m³ (Tested)	Lab Ref No :	CH9468/10			
Source :	<b>TAC170</b>		Client Ref No :	TAC170			

			Т	est Results				
Maximum dry de	nsity	2.14	t/m³		Natural wat	er content	0.3	%
Optimum water	content	6.0	%		Fraction tes	sted	Passing 19.0m	m
Sample ID		+2%	+4%	+6%	+8%	+10%	+12%	
Bulk density	t/m³	2.106	2.191	2.272	2.268	2.257	2.206	
Water content	%	2.2	4.1	6.1	7.3	8.9	10.7	
Dry density	t/m³	2.061	2.104	2.142	2.114	2.072	1.994	
Sample condition	٦	Moist	Wet	Wet	Wet	Wet	Saturated	
		Firm	Firm	Firm	Firm	Soft	Soft	



Notes Test Methods Compaction NZS 4402 : 1986 Test 4.1.1 (Standard) All information supplied by Client

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Date tested : 28 November 2022 Date reported : 30 November 2022 Sampling is not covered by IANZ Accreditation. Results apply only to sample tested. This report may only be reproduced in full

IANZ Approved Signatory

Date :

Designation : Laboratory Manager 30 November 2022

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Page 2 of 4

Project :	Barrytown Mineral Sands Ta	ilings	
Location :	<b>Barrytown West Coast</b>		
Client :	<b>Resource Development Consu</b>	iltants Limited	_
Contractor :	<b>Resource Development Consu</b>	iltants Limited	
Sampled by :	Tom Bunny		
Date sampled :	17 October 2022		
Sampling method :	NZS 4402: 1986 (Coarse)		
Sample description :	Gravelly SAND with some sil	t and trace clay	
Sample condition :	Dry as received	Project No :	6-JRESD.16/6LC
Source:	<b>TAC170</b>	Lab Ref No :	CH9468/10
		<b>Client Ref No:</b>	<b>TAC170</b>

# Test Results

Solid Density (t/m<sup>3</sup>):

2.82

Test Method: Determination of the Solid Density of Soil Particles NZS 4402 : 1986 : Test 2.7.1 (Passing 19.0mm)

Date tested : 2 Date reported : 2

25 November 2022 2 December 2022

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Str

Designation : Date : *Laboratory Manager* 2 December 2022

PF-LAB-004 ( 30/05/2013)

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PLASTICITY INDEX FOR AGGREGATES
TEST REPORT

Project :	Barrytown Mineral Sands Tailing	JS	
Location :	Barrytown West Coast		
Client:	Resource Development Consult	ants Limited	
Contractor :	Resource Development Consult	ants Limited	
Sampled by :	Tom Bunny		
Date sampled :	17 October 2022		
Sampling method :	NZS 4402: 1986 (Coarse)		
Sample description :	Gravelly SAND with some silt an	d trace caly	
Sample condition :	As Received	Project No :	6-JRESD.16/6LC
Source :	TAC170	Lab Ref No :	CH9468/10
		Client Ref No :	TAC170

	Test Results	
Client Ref No :	TAC170	
Cone penetration limit :	19	
Plastic limit :	Unable to Roll Threads	
Plasticity index :	NP	
Sample fraction :	Fraction passing 425µm test sieve	
As received water content :	0.3	
Fest Methods		

20 / / 00 - 0015 T - 1 - 7 - 1	
ZS 4407 : 2015 Test 3.1	
ZS 4407 : 2015 : Test 3.2	
ZS 4407 : 2015 : Test 3.3	
ZS 4407 : 2015 : Test 3.4	
12	IZS 4407 : 2015 : Test 3.2 IZS 4407 : 2015 : Test 3.3 IZS 4407 : 2015 : Test 3.3

Date tested :	1 December 2022
Date reported :	2 December 2022

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IANZ Approved Signatory

Designation : Laboratory Manager Date :

PF-LAB-053 (09/06/2021)

815 2 December 2022



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					]			
oject :	Barrytown M	lineral San	ds Tailinas					
ocation :	Barrytown W		5					
ient :	Resource De		t Concultan					
		velopmen	COnsultan	ts Linned				
ient/Sample Ref :	TAC176							
ontractor :	Resource De	velopmen	t Consultan	ts Limited				
mpled by :	Tom Bunny							
ate received :	7 November	2022						
mpling method :	NZS 4402:198	36 (Coarse	)					
mple condition :	Damp as rec	eived				Project No:	6-JRESD.16/6L	С
mple description :	SAND wit mi	nor silt, tra	acce gravel a	and clay		Lab Ref No:	CH9468/11	
lid Particle Density (t/		2.80	Tested			Client Ref:	TAC176	
ater Content (as rece		0.0	%					
	Sieve Ana		70			Hydromete	er Analysis	
Sieve Size Passing	Sieve Size	Passing	Sieve Size	Passing	Particle Size	Passing	Particle Size	Passing
(mm) (%)	(mm)	(%)	(mm)	(%)	(mm)	(%)	(mm)	(%)
63.0	4.75	98	0.300	87	0.0475	9	0.0071	3
37.5 100	2.36	97	0.212	42	0.0346	7	0.0051	2
19.0 100	1.18	97	0.150	21	0.0250	7	0.0037	2
13.2 100	0.600	96	0.075	10	0.0182	6	0.0028	1
9.5 99	0.425	95	0.063	9	0.0135	5	0.0015	1
Note: "" denotes s	ieve not used and/	or hydromet	ter analysis not	tested	0.0098	4		
90 80 70 60 50 40 30 20 10 0.001 fine	0.010	0 Darse	.100	Size (mm) 1.00	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	10.000	m coarse	100.000 very coarse
CLAY	SILT		S	AND		GRAV	EL	Toodioo
t Methods					Notes			
ticle Size Analysis: NZS 4402:19	986: Test 2.8.4 (Washe	ed Grading & H	lydrometer Met	nod)		1. A. A. S. A.		
					All information s			
			Sampling is n	ot covered by	IANZ Accredita	tion. Results a	pply only to san	nple tested
te Tested:	29 November	2022	This report m	ay only be rep	roduced in full			
te Reported:	1 December 2	2022	-		ACCRED	Te	st results indicated	
NZ Approved Signa	tory oly					SCI SCI	ope of the labora	
		1anager				ac	creditation	

#### IANZ Approved Signatory Designation :

Date :

PF-LAB-100 (11/07/2020)

1 December 2022

52C Hayton Rd PO Box 1482, Christchurch Mail Centre, 8140, Christchurch, New Zealand

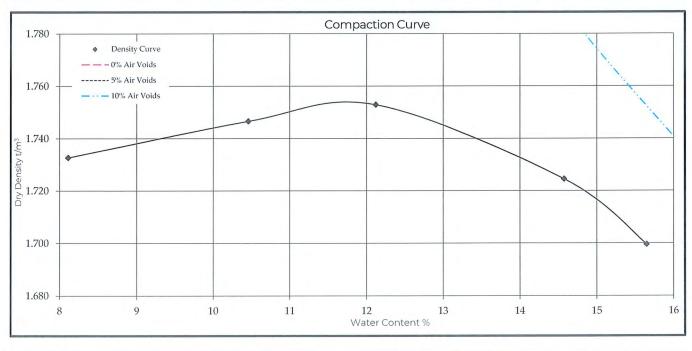
FSTING LABORATO

Page 1 of 5

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Project :	Barrytown Mineral Sands T	ailings					
Location :	Barrytown West Coast						
Client :	Resource Development Consultants Limited						
Contractor :	Resource Development Co	Resource Development Consultants Limited					
Sampled by :	Tom Bunny						
Date sampled :	17 October 2022						
Sampling method :	NZS 4402: 1986 (Coarse)						
Sample description :	SAND with minor silt, trace	e gravel and clay					
Sample condition :	Damp as received	Project No :	6-JRESD.16/6LC				
Solid density :	2.80 t/m <sup>3</sup> (Tested)	Lab Ref No :	CH9468/11				
Source :	TAC176	Client Ref No:	TAC176				

			Т	est Results				
Maximum dry de	ensity	1.75	t/m <sup>3</sup>		Natural wat	er content	0.0	%
Optimum water	content	12.0	%		Fraction tested Passing 19.0mm			nm
Sample ID		+8%	+10%	+12%	+14%	+16%	+18%	
Bulk density	t/m <sup>3</sup>	1.873	1.929	1.965	1.976	1.966	1.904	
Water content	%	8.1	10.5	12.1	14.6	15.6	18.1	
Dry density	t/m³	1.733	1.747	1.753	1.725	1.700	1.611	
Sample condition	า	Wet	Wet	Wet	Wet	Wet	Saturated	
		Firm	Firm	Firm	Firm	Soft	Soft	



Test Methods Notes Compaction NZS 4402 : 1986 Test 4.1.1 (Standard) All information supplied by Client

52C Hayton Rd

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8140, Christchurch, New Zealand

Date tested : 28 November 2022 Date reported : 1 December 2022

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Str

Date :

Designation: Laboratory Manager 1 December 2022

PF-LAB-026 (10/07/20)

WSP

Christchurch (Hayton Rd) Quality Management Systems Certified to ISO 9001



Test results indicated as not accredited are outside the scope of the laboratory's accreditation

Page 2 of 5

	ST REPORT		SI)	
Project :	Barrytown Mineral Sands Tailin	ngs		
Location :	<b>Barrytown West Coast</b>			
Client :	Resource Development Consulta	ants Limited		
Contractor :	Resource Development Consulta	ants Limited		
Sampled by :	Tom Bunny			
Date sampled :	17 October 2022			
Sampling method :	NZS 4402: 1986 (Coarse)			
Sample description :	SAND with minor silt, trace gra	vel and clay		
Sample condition :	Dry as received	<b>Project No :</b>	6-JRESD.16/6LC	
Source:	<b>TAC176</b>	Lab Ref No :	CH9468/11	
		<b>Client Ref No :</b>	<b>TAC176</b>	-

5	Tes	t Results
	Solid Density (t/m <sup>3</sup> ):	2.80
	Test Method: Determination of the Solid De (Passing 19.0mm)	ensity of Soil Particles NZS 4402 : 1986 : Test 2.7.1
	(Passing 19.0mm)	
Date tested :	1 December 2022	

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Approved

SIG

Date reported : 2 December 2022

Designation : Date :

Laboratory Manager 2 December 2022

PF-LAB-004 ( 30/05/2013)

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52C Hayton Rd, Wigram PO Box 1482, Christchurch Mail Centre, Christchurch 8140, New Zealand

Page 3 of 5

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#### PLASTICITY INDEX FOR AGGREGATES **TEST REPORT**

Project :	Barrytown Mineral Sands Tailing	JS	
Location :	Barrytown West Coast		
Client :	Resource Development Consult	ants Limited	
Contractor :	Resource Development Consult	ants Limited	
Sampled by :	Tom Bunny		
Date sampled :	17 October 2022		
Sampling method :	NZS 4402: 1986 (Coarse)		
Sample description :	SAND with minor silt, trace grav	el and clay	
Sample condition :	As Received	Project No :	6-JRESD.16/6LC
Source :	TAC176	Lab Ref No :	CH9468/11
		Client Ref No :	TAC176

And the second s	Test Results	
Client Ref No :	TAC176	
Cone penetration limit :	30	
Plastic limit :	Unable to Roll Threads	
Plasticity index :	NP	
Sample fraction :	Fraction passing 425µm test sieve	
As received water content :	0	
Test Methods		
Water Content NZS	4407 : 2015 Test 3.1	

rest methous		
Water Content	NZS 4407 : 2015 Test 3.1	
Cone Penetration	NZS 4407 : 2015 : Test 3.2	
Plastic Limit	NZS 4407 : 2015 : Test 3.3	
Plasticity Index	NZS 4407 : 2015 : Test 3.4	
Plasticity Index	NZS 4407 : 2015 : Test 3.4	

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Date tested :	30 November 2022
Date reported :	2 December 2022

Designation : Laboratory Manager

Date :

PF-LAB-053 (09/06/2021)



IANZ Approved Signatory

2 December 2022



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Telephone +64 3 343 0739 Website www.wsp.com/nz

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#### LINEAR SHRINKAGE TEST REPORT

Project :	Barrytown Mineral Sands Tailings
Location :	Barrytown West Coast
Client :	Resource Development Consultants Limited
Contractor :	Resource Development Consultants Limited
Sampled by :	Tom Bunny
Date sampled :	17 October 2022
Sampling method :	NZS 4402: 1986 (Coarse)
Sample description :	SAND with minor silt, trace gravel and clay
Sample condition :	Dry as received
Source:	TAC176

# vsp

Project No : 6-JRESD.16/6LC Lab Ref No : CH9468/11 Client Ref No : TAC176

	Tes	t Results
Linear Shrinkage (%):	o	
Test Methods		Notes
Linear Shrinkage	NZS 4402 : 1986, Test 2.6	Materials used: Passing 425um sieve

Date tested : Date reported :

Designation :

PF-LAB-101 (30/05/2013)

Date :

IANZ Approved Signatory

30 November 2022 2 December 2022

Laboratory Manager

2 December 2022

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roject :		Barrytown I	Mineral Sar	nds Tailings					
ocation :		Barrytown West Coast							
lient :									
lient/Samp	le Ref ·	TSON002	e reielennen						
ontractor :	ie ner.		evelopmer	nt Consultan	ts limited				
ontractor.		Resource D	evelopiner	it consultan	Li L				
ampled by	:	Tom Bunny	1						
ate receive	ed :	7 Novembe	r 2022						
ampling m	ethod :	NZS 4402:19	986 (Coarse	e)					
ample cond	dition :	Damp as re	ceived				Project No:	6-JRESD.16/6L	С
ample desc	cription :	SAND with	some grave	el minor silt a	and trace c	lay	Lab Ref No:	CH9468/1	
olid Particle	e Density (t/ı	m <sup>3</sup> ):	2.92	Tested			Client Ref:	TSON002	
	ent (as recei		0.1	%					
		Sieve An	alysis				Hydromete	er Analysis	_
Sieve Size	Passing	Sieve Size	Passing	Sieve Size	Passing	Particle Size	Passing	Particle Size	Passing
(mm)	(%)	(mm)	(%)	(mm)	(%)	(mm)	(%)	(mm)	(%)
63.0		4.75	86	0.300	77	0.0430	10	0.0068	3
37.5	100	2.36	83	0.212	62	0.0315	9	0.0049	3
19.0	99	1.18	82	0.150	36	0.0232	8	0.0035	2
13.2 9.5	96 92	0.600	80 79	0.075	11	0.0170	7	0.0027	2
		eve not used and		11		0.0093	4	0.0015	
Note:			0.063	Sieve Apert			9.5	37.5	63.0
100 90 00 00 00 00 00 00 00 00 00 00 00 0	fine	0.010 medium		Sieve Apert	Size (mm) 1.0		10.000		0.000 Very coarse
100 90 80 60 40 40 40 10 0 0 0 0	fine			Sieve Apert	Size (mm) 1.0	00	10.000	Im coarse	100.000
100 90 80 60 60 40 40 20 10 0 0 0.001	fine	medium		Sieve Apert	Size         Image: Constraint of the second se	00	10.000 ne mediu	Im coarse	100.000
100 90 80 70 50 40 10 0 0.001 CLAY St Methods		medium SILT		Sieve Apert	Size (mm) 1.0 edium cc AND	00	10.000 ne mediu	Im coarse	100.000
100 90 80 70 50 40 10 0 0.001 CLAY St Methods		medium		Sieve Apert	Size (mm) 1.0 edium cc AND	Provide     Provide	10.000 ne mediu	im coarse	100.000
100 90 80 70 50 40 10 0 0.001 CLAY St Methods		medium SILT		Sieve Apert	size (mm) 1.0 edium cc	Notes	10.000 ne mediu GRAVI	m coarse	100.000
100 90 80 80 80 70 90 90 90 90 90 90 90 90 90 90 90 90 90	ysis: NZS 4402:19	Medium SILT 986: Test 2.8.4 (Was	coarse	Sieve Apert	size (mm) 1.0 edium co AND	Notes All information :	10.000 ne mediu GRAVI	m coarse	100.000 Very coarse
100 90 80 70 see 60 40 30 20 10 0.001 CLAY st Methods rticle Size Analy	ysis: NZS 4402:19	medium SILT 186: Test 2.8.4 (Was 14 Novembe	coarse	Sieve Apert	size (mm) 1.0 edium co AND	Notes	10.000 ne mediu GRAVI	m coarse	100.000 Very coarse
100 90 80 70 90 80 70 90 80 70 90 90 90 90 90 90 90 90 90 9	ysis: NZS 4402:19	medium SILT 186: Test 2.8.4 (Was 14 November 21 November	coarse	Sieve Apert	size (mm) 1.0 edium co AND	Notes All information :	10.000 ne mediu GRAVI	m coarse EL  tresults indicate credited are out ope of the labora	100.000 very coarse
100 90 80 70 80 70 60 90 80 70 90 80 70 90 80 70 90 80 70 90 90 80 70 90 90 90 90 90 90 90 90 90 9	ed:	medium SILT 186: Test 2.8.4 (Was 14 November 21 November	coarse	Sieve Apert	size (mm) 1.0 edium co AND	Notes All information strong of the full strong of the strong of the strong str	10.000 ne mediu GRAVI	im coarse EL EL pply only to san	100.000 very coarse

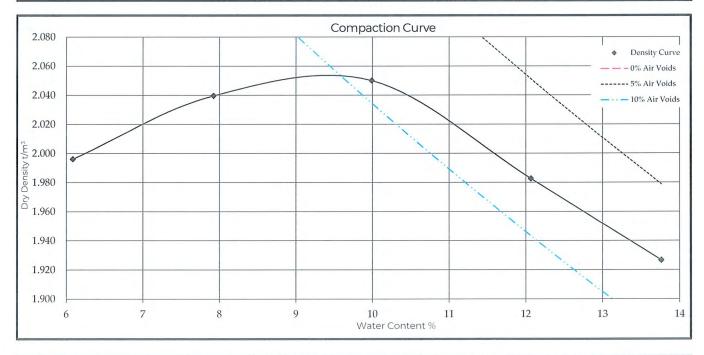
PF-LAB-100 (11/07/2020)

Christchurch (Hayton Rd) Quality Management Systems Certified to ISO 9001 52C Hayton Rd PO Box 1482, Christchurch Mail Centre, 8140, Christchurch, New Zealand Page 1 of 5

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Project :	Barrytown Mineral Sands Tailings					
Location :	Barrytov	wn West Coast				
Client :	Resourc	e Development Consu	ultants Limited			
Contractor :	Resourc	e Development Consu	ultants Limited			
Sampled by :	Tom Bu	nny				
Date sampled :	17 Octob	per 2022				
Sampling method :	NZS 440	02: 1986 (Coarse)				
Sample description :	SANDW	ith some gravel mino	r silt and trace clay			
Sample condition :	Damp a	s received	Project No :	6-JRESD.16/6LC		
Solid density :	2.92	t/m <sup>3</sup> (Tested)	Lab Ref No :	CH9468/1		
Source :	TSONOC	)2	Client Ref No :	TSON002		

			Т	est Results				
Maximum dry de	ensity	2.05	t/m <sup>3</sup>		Natural wat	er content	0.1	%
Optimum water	content	10.0	%		Fraction tes	sted F	Passing 19.0	mm
Sample ID		+6%	+8%	+10%	+12%	+14%		
Bulk density	t/m <sup>3</sup>	2.118	2.201	2.255	2.222	2.192		
Water content	%	6.1	7.9	10.0	12.1	13.8		
Dry density	t/m <sup>3</sup>	1.996	2.040	2.050	1.983	1.927		
Sample condition	า	Moist	Wet	Wet	Wet	Saturated		
		Hard	Firm	Firm	Firm	Soft		



Test Methods Notes Compaction NZS 4402 : 1986 Test 4.1.1 (Standard) All information supplied by Client

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8140, Christchurch, New Zealand

Date tested : 14 November 2022 Date reported : 21 November 2022

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Sty

Date :

Designation : Laboratory Manager 21 November 2022

PF-LAB-026 (10/07/20)

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Page 2 of 5

Project :	Barrytown Mineral Sands Tai	lings	
Location :	<b>Barrytown West Coast</b>		
Client :	<b>Resource Development Consu</b>	Itants Limited	
Contractor :	<b>Resource Development Consu</b>	Itants Limited	
Sampled by :	Tom Bunny		
Date sampled :	17 October 2022		
Sampling method :	NZS 4402: 1986 (Coarse)		
Sample description :	SAND with some gravel minor	r silt and <u>trace clay</u>	
Sample condition :	Dry as received	<b>Project No :</b>	6-JRESD.16/6LC
Source:	TSON002	Lab Ref No :	CH9468/1
		<b>Client Ref No :</b>	TSON002

#### **Test Results**

2.92

Solid Density (t/m<sup>3</sup>):

Test Method: Determination of the Solid Density of Soil Particles NZS 4402 : 1986 : Test 2.7.1 (Passing 19.0mm)

Date tested :

16 November 2022

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Date reported : 21 November 2022

Approved

4

Designation : Date :

Laboratory Manager 21 November 2022

PF-LAB-004 ( 30/05/2013)

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#### PLASTICITY INDEX FOR AGGREGATES **TEST REPORT**

Project :	Barrytown Mineral Sands Tailings				
Location :	Barrytown West Coast				
Client :	Resource Development Consultants Limited				
Contractor:	Resource Development Consultants	Limited			
Sampled by :	Tom Bunny				
Date sampled :	17 October 2022				
Sampling method :	NZS 4402: 1986 (Coarse)				
Sample description :	SAND with some gravel minor silt and	d trace clay			
Sample condition :	As Received	Project No :	6-JRESD.16/6LC		
Source :	TSON002	Lab Ref No :	CH9468/1		
		Client Ref No :	TSON002		

Test Results	
TSON002	Client Ref No :
24	Cone penetration limit :
Unable to Roll Threads	Plastic limit :
NP	Plasticity index :
Fraction passing 425µm test sieve	Sample fraction :
0.1	As received water content :
	Test Methods

Water Content	NZS 4407 : 2015 Test 3.1	
Cone Penetration	NZS 4407 : 2015 : Test 3.2	
Plastic Limit	NZS 4407 : 2015 : Test 3.3	
Plasticity Index	NZS 4407 : 2015 : Test 3.4	

Date tested :	16 November 2022
Date reported	: 22 November 2022

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#### IANZ Approved Signatory

Date :

PF-LAB-053 (09/06/2021)

Stil Designation : Laboratory Manager 22 November 2022



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#### LINEAR SHRINKAGE TEST REPORT

Project :	Barrytown Mineral Sands Tailings	
Location :	Barrytown West Coast	
Client :	Resource Development Consultants Limited	
Contractor :	Resource Development Consultants Limited	
Sampled by :	Tom Bunny	
Date sampled :	17 October 2022	
Sampling method :	NZS 4402: 1986 (Coarse)	
Sample description :	SAND with some gravel minor silt and trace clay	
Sample condition :	Dry as received	F
Source:	TSON002	L

Project No :	6-JRESD.16/6LC	
Lab Ref No :	CH9468/1	
Client Ref No :	TSON002	

	Tes	st Results
Linear Shrinkage (%):	C	
Test Methods		Notes
Linear Shrinkage	NZS 4402 : 1986, Test 2.6	Materials used: Passing 425um sieve
Date tested : 16 Nove	ember 2022 Sampli	ng is not covered by IANZ Accreditation. Results apply only to sample
Date reported : 22 Nove		port may only be reproduced in full



Designation : Date :

PF-LAB-101 (30/05/2013)

on : Laboratory Manager 22 November 2022

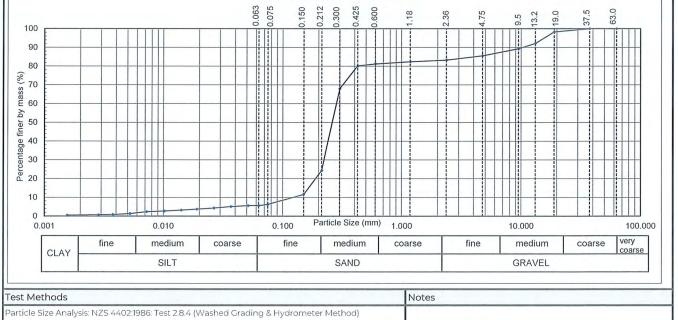


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						_		
Project :		Barrytown	Mineral Sar	nds Tailings				
Location :		Barrytown	West Coast					
Client :		Resource D	evelopmer	nt Consultan	ts Limited			
Client/Samp	ole Ref :	TSON003						
Contractor :		Resource D	evelopmer	nt Consultan	ts Limited			
Sampled by	:	Tom Bunny	,					
Date receive	ed :	7 Novembe	r 2022					
Sampling m	nethod :	NZS 4402:19	986 (Coarse	2)				
Sample con	dition :	Damp as re	ceived				Project No:	6-JRESD.16/6LC
Sample des	cription :	SAND with	some grave	el and minor	silt		Lab Ref No:	CH9468/2
Solid Particl	e Density (t/	<sup>2</sup> m <sup>3</sup> ):	2.76	Tested			Client Ref:	TSON003
Water Cont			0.2	%				
		Sieve Ar	alysis				Hydromet	er Analysis
Sieve Size	Passing	Sieve Size	Passing	Sieve Size	Passing	Particle Size	Passing	Particle Size
(mm)	(%)	(mm)	(%)	(mm)	(%)	(mm)	(%)	(mm)
63.0		4.75	85	0.300	68	0.0513	6	0.0072
37.5	100	2.36	83	0.212	24	0.0368	5	0.0052
19.0	98	1.18	82	0.150	12	0.0265	4	0.0037
13.2	92	0.600	81	0.075	6	0.0191	4	0.0028
9.5	89	0.425	80	0.063	6	0.0141	3	0.0015
Note:	"" denotes si	ieve not used and	d/or hydrome	ter analysis not	tested	0.0101	3	
				Sieve Aper	ure Size (m	m)		
100			0.063	•	0.600 0.425	2.36	4.75 9.5 13.2	19.0 .37.5 .63.0



Date Tested:

14 November 2022

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22 November 2022

IANZ Approved Signatory

SK

Laboratory Manager

22 November 2022

Date Reported:

Designation :

PF-LAB-100 (11/07/2020)

Date :

CREDITED

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TO

Test results indicated as not accredited are outside the scope of the laboratory's accreditation

Passing

(%)

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Pagel of 4

WSP Christchurch (Hayton Rd) Quality Management Systems Certified to ISO 9001

52C Hayton Rd PO Box 1482, Christchurch Mail Centre, 8140, Christchurch, New Zealand



Project :	Barrytown Mineral Sands Tail	ings	
Location :	Barrytown West Coast		
Client :	Resource Development Cons	ultants Limited	
Contractor :	Resource Development Cons	ultants Limited	
Sampled by :	Tom Bunny		
Date sampled :	17 October 2022		
Sampling method :	NZS 4402: 1986 (Coarse)		
Sample description :	SAND with some gravel mino	r silt and trace clay	
Sample condition :	Damp as received	Project No :	6-JRESD.16/6LC
Solid density :	2.76 t/m <sup>3</sup> (Tested)	Lab Ref No :	CH9468/2
Source :	TSON003	Client Ref No :	TSON003

			Т	est Results				
Maximum dry de	ensity	1.88	t/m <sup>3</sup>		Natural wate	er content	0.2	%
Optimum water	content	10.0	%	Fraction tested Passing 19.0mm			im	
Sample ID		+6%	+8%	+10%	+12%	+14%	+16%	
Bulk density	t/m <sup>3</sup>	1.897	1.952	2.044	2.102	2.060	1.977	
Water content	%	6.0	7.9	9.8	11.9	13.1	15.8	
Dry density	t/m³	1.790	1.809	1.862	1.879	1.822	1.707	
Sample condition	า	Moist	Wet	Wet	Wet	Wet	Saturated	
		Hard	Firm	Firm	Firm	Soft	Soft	

1.900				С	ompaction	Curve				
						N.				Density Curv
1.880						- in			··	– 0% Air Voids
1.860				~						5% Air Voids
1.840							1			- 10% Air Voic
1.820								N.		1
1.820       1.800       1.780								1		
1.780									···	
1.760									1.	
1.740										1.
										1.
1.720										
1.700										•
5	6	7	8	9	10 1 Water Co		2 1	3 14	1	5

Test Methods Notes NZS 4402 : 1986 Test 4.1.1 (Standard) All information supplied by Client Compaction

Date tested : 14 November 2022 Date reported : 22 November 2022 Sampling is not covered by IANZ Accreditation. Results apply only to sample tested. This report may only be reproduced in full

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Sil

Designation : Laboratory Manager Date :

22 November 2022

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PF-LAB-026 (10/07/20)

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Page 2 of 4

TE	ST REPORT		SI
Project :	Barrytown Mineral Sands Taili	ngs	
Location :	<b>Barrytown West Coast</b>		
Client :	<b>Resource Development Consult</b>	ants Limited	-
Contractor :	<b>Resource Development Consult</b>	ants Limited	
Sampled by :	Tom Bunny		
Date sampled :	17 October 2022		
Sampling method :	NZS 4402: 1986 (Coarse)		
Sample description :	SAND with some gravel and mi	nor silt	
Sample condition :	Dry as received	<b>Project No :</b>	6-JRESD.16/6LC
Source:	TSON003	Lab Ref No :	CH9468/2
		<b>Client Ref No :</b>	TSON003

# **Test Results**

2.76

Solid Density (t/m<sup>3</sup>):

Test Method: Determination of the Solid Density of Soil Particles NZS 4402 : 1986 : Test 2.7.1 (Passing 19.0mm)

17 November 2022 Date tested : Date reported : 22 November 2022

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Sti Laboratory Manager

Designation : Date :

22 November 2022

PF-LAB-004 ( 30/05/2013)

WSP Opus Christchurch Laboratory Quality Management Systems Certified to ISO 9001

52C Hayton Rd, Wigram PO Box 1482, Christchurch Mail Centre, Christchurch 8140, New Zealand

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Facsimile Website www.wspopus.co.nz

#### PLASTICITY INDEX FOR AGGREGATES **TEST REPORT**

Project :	Barrytown Mineral Sands Tailir	ngs			
Location :	Barrytown West Coast				
Client :	Resource Development Consultants Limited				
Contractor :	Resource Development Consu	Itants Limited			
Sampled by :	Tom Bunny				
Date sampled :	17 October 2022				
Sampling method :	NZS 4402: 1986 (Coarse)				
Sample description :	SAND with some gravel minor	silt and trace clay			
Sample condition :	As Received	Project No :	6-JRESD.16/6LC		
Source :	TSON002	Lab Ref No :	CH9468/2		
		Client Ref No :	TSON003		

Test Results	
TSON003	
33	
Unable to Roll Threads	
NP	
Fraction passing 425µm test sieve	
0.2	
	TSON003 33 Unable to Roll Threads NP Fraction passing 425µm test sieve

NZS 4407 : 2015 Test 3.1	
NZS 4407 : 2015 : Test 3.2	
NZS 4407 : 2015 : Test 3.3	
NZS 4407 : 2015 : Test 3.4	
	NZS 4407 : 2015 : Test 3.2 NZS 4407 : 2015 : Test 3.3

Date tested : 15 November 2022 Date reported : 22 November 2022

Sampling is not covered by IANZ Accreditation. Results apply only to sample tested. This report may only be reproduced in full All information supplied by Client

# IANZ Approved Signatory

Date :

PF-LAB-053 (09/06/2021)

Sin Designation: Laboratory Manager 22 November 2022



Test results indicated as not accredited are outside the scope of the laboratory's accreditation

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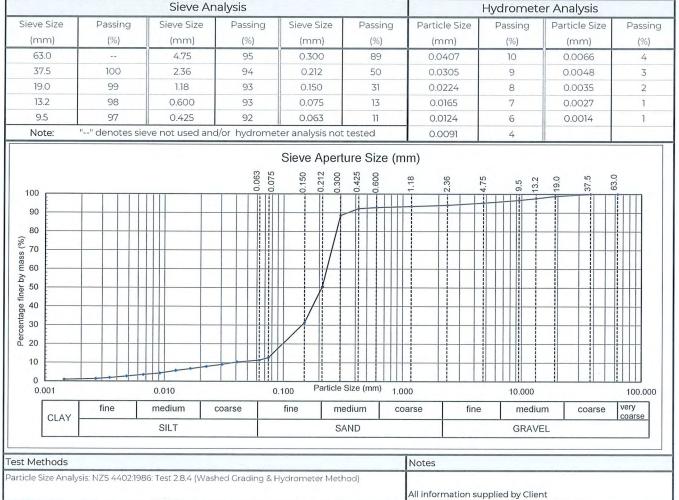
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### PARTICLE SIZE ANALYSIS (HYDROMETER METHOD) **TEST REPORT**

Project :	Barrytown Mineral Sands Tailings
Location :	Barrytown West Coast
Client :	Resource Development Consultants Limited
Client/Sample Ref :	TSON004
Contractor :	Resource Development Consultants Limited

Sampled by: Tom Bunny Date received : 7 November 2022 Sampling method : NZS 4402:1986 (Coarse) Sample condition : Damp as received Sample description : SAND with some gravel, silt and trace clay Solid Particle Density (t/m<sup>3</sup>): 2.96 Tested Water Content (as received): % 0.1

6-JRESD.16/6LC Project No: Lab Ref No: CH9468/3 Client Ref: TSON004



Date Tested:

14 November 2022

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Designation :

Date :

Date Reported:

22 November 2022

Sty

Laboratory Manager

22 November 2022

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PF-LAB-100 (11/07/2020)

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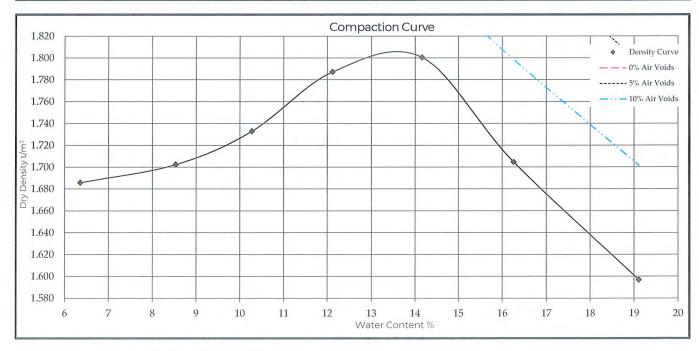
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# DRY DENSITY / WATER CONTENT RELATIONSHIP

	STANDARD COMPACTION		
Project :	Barrytown Mineral Sands Taili	ngs	
Location :	Barrytown West Coast		
Client :	Resource Development Consu	ultants Limited	
Contractor :	Resource Development Consu	ultants Limited	
Sampled by :	Tom Bunny		
Date sampled :	17 October 2022		
Sampling method :	NZS 4402: 1986 (Coarse)		
Sample description :	SAND with minor gravel, silt a	nd trace clay	
Sample condition :	Damp as received	Project No :	
Solid density :	2.96 t/m <sup>3</sup> (Tested)	Lab Ref No :	
Source :	TSON004	Client Ref No :	

			Т	est Results				
Maximum dry de	ensity	1.80	t/m <sup>3</sup>		Natural wate	er content	0.1	%
Optimum water content		14.0 %		Fraction tested P		Passing 19.0m	m	
Sample ID		+6%	+8%	+10%	+12%	+14%	+16%	+18%
Bulk density	t/m³	1.793	1.848	1.911	2.004	2.055	1.982	1.902
Water content	%	6.4	8.5	10.3	12.1	14.2	16.3	19.1
Dry density	t/m³	1.686	1.702	1.733	1.787	1.801	1.705	1.597
Sample conditio	n	Moist	Wet	Wet	Wet	Wet	Wet	Saturated
		Hard	Firm	Firm	Firm	Firm	Soft	Soft



Test Methods Notes NZS 4402 : 1986 Test 4.1.1 (Standard) All information supplied by Client Compaction

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Date tested : 17 November 2022 Date reported : 22 November 2022 Sampling is not covered by IANZ Accreditation. Results apply only to sample tested. This report may only be reproduced in full

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### IANZ Approved Signatory

Din

Date :

Designation : Laboratory Manager 22 November 2022

PF-LAB-026 (10/07/20)

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Test results indicated as not

6-JRESD.16/6LC

CH9468/3

TSON004

### SOLID DENSITY OF SOIL PARTICLES **TEST REPORT**

Barrytown Mineral Sands Tai	ilinos				
	Itants Limited				
Tom Bunny					
17 October 2022					
NZS 4402: 1986 (Coarse)					
SAND with minor gravel, silt :	and trace clay				
Dry as received	Project No :	6-JRESD.16/6LC			
TSON004	Lab Ref No :	CH9468/3			
	<b>Client Ref No :</b>	TSON004			
	Barrytown West Coast Resource Development Consu Resource Development Consu Tom Bunny 17 October 2022 NZS 4402: 1986 (Coarse) SAND with minor gravel, silt a Dry as received	Resource Development Consultants LimitedResource Development Consultants LimitedTom Bunny17 October 2022NZS 4402: 1986 (Coarse)SAND with minor gravel, silt and trace clayDry as receivedTSON004			

1161

	Test	t Results	
	Solid Density (t/m <sup>3</sup> ):	2.96	
	Test Mathed Determination of the Calib		
	(Passing 19.0mm)	ensity of Soil Particles NZS 4402 : 1986 : Test 2.7.1	
ata tastad y	18 November 2022		

Date tested :

18 November 2022 Date reported : 22 November 2022

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Approved

812

Designation : Date :

Laboratory Manager 22 November 2022

PF-LAB-004 ( 30/05/2013)

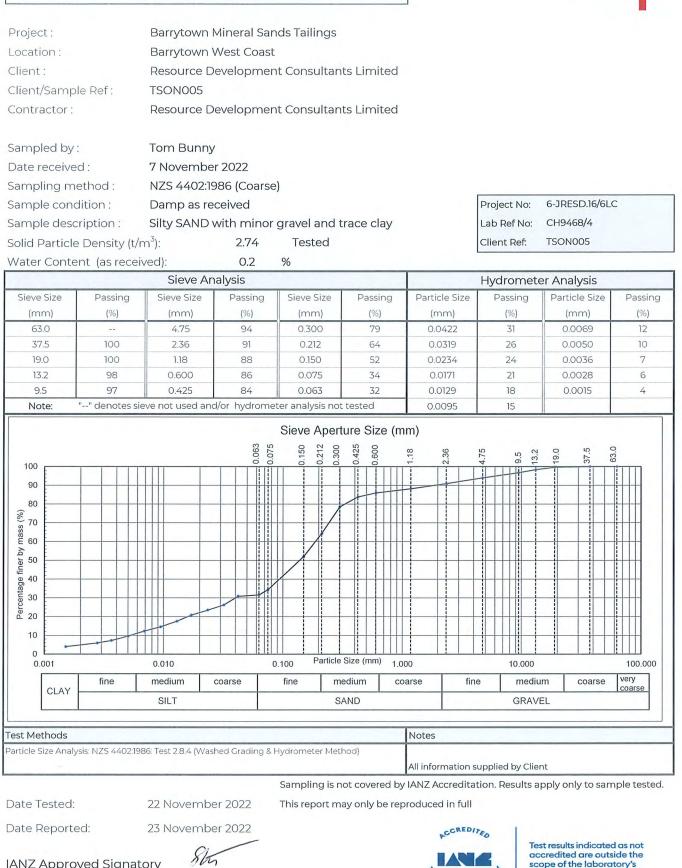
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### PARTICLE SIZE ANALYSIS (HYDROMETER METHOD) **TEST REPORT**



IANZ Approved Signatory

Designation : Date :

PF-LAB-100 (11/07/2020)

Sta

Laboratory Manager

23 November 2022

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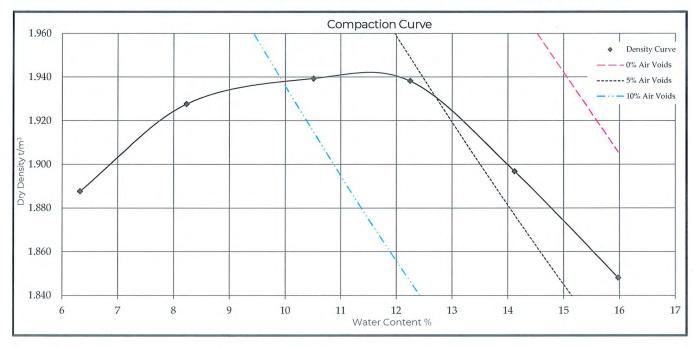
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accreditation

### DRY DENSITY / WATER CONTENT RELATIONSHIP STANDARD COMPACTION

Project :	Barryto	wn Mineral Sands Taili	ngs	
Location :	Barrytov	wn West Coast		
Client :	Resourc	e Development Consu	ultants Limited	
Contractor :	Resourc	e Development Consu	ultants Limited	
Sampled by :	Tom Bu	nny		
Date sampled :	17 Octob	per 2022		
Sampling method :	NZS 440	02: 1986 (Coarse)		
Sample description :	Silty SAM	ND witrh minor gravel	and trace clay	
Sample condition :	Damp a	s received	Project No :	6-JRESD.16/6LC
Solid density :	2.74	t/m <sup>3</sup> (Tested)	Lab Ref No :	CH9468/4
Source :	TSONOC	05	Client Ref No :	TSON005

			Т	est Results					
Maximum dry de	nsity	1.94	t/m <sup>3</sup>		Natural wat	er content	0.2	%	
		12.0	%		Fraction tested Pass		Passing 19.0m	sing 19.0mm	
Sample ID		+6%	+8%	+10%	+12%	+14%	+16%		
Bulk density	t/m³	2.007	2.086	2.143	2.176	2.165	2.143		
Water content	%	6.3	8.2	10.5	12.2	14.1	16.0		
Dry density	t/m³	1.888	1.928	1.939	1.938	1.897	1.848		
Sample condition	٦	Moist	Wet	Wet	Wet	Wet	Wet		
		Hard	Firm	Firm	Firm	Firm	Saturated		



Test Methods Notes NZS 4402 : 1986 Test 4.1.1 (Standard) All information supplied by Client Compaction

52C Hayton Rd

PO Box 1482, Christchurch Mail Centre,

8140, Christchurch, New Zealand

Date tested : 18 November 2022 Date reported : 26 November 2022 Sampling is not covered by IANZ Accreditation. Results apply only to sample tested. This report may only be reproduced in full

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Date :

815 Designation: Laboratory Manager 26 November 2022

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WSP Christchurch (Hayton Rd)

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### SOLID DENSITY OF SOIL PARTICLES TEST REPORT

Project :	Barrytown Mineral Sands Tai	lings	
Location :	<b>Barrytown West Coast</b>		
Client :	<b>Resource Development Consu</b>	Itants Limited	
Contractor :	<b>Resource Development Consu</b>	ltants Limited	
Sampled by :	Tom Bunny		
Date sampled :	17 October 2022		
Sampling method :	NZS 4402: 1986 (Coarse)		
Sample description :	Slity SAND with minor gravel	and trace clay	
Sample condition :	Dry as received	Project No :	6-JRESD.16/6LC
Source:	TSON005	Lab Ref No :	CH9468/4
		<b>Client Ref No:</b>	TSON005

# Test Results Solid Density (t/m³): 2.74 Test Method: Determination of the Solid Density of Soil Particles NZS 4402 : 1986 : Test 2.7.1 (Passing 19.0mm)

Date tested : Date reported :

18 November 2022 26 November 2022

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Approved

815

Designation : Date : Laboratory Manager 26 November 2022

PF-LAB-004 (30/05/2013)

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### PLASTICITY INDEX FOR AGGREGATES TEST REPORT

Project :	Barrytown Mineral Sands Taili	ngs	
Location :	Barrytown West Coast		
Client :	Resource Development Cons	ultants Limited	
Contractor :	Resource Development Cons	ultants Limited	
Sampled by :	Tom Bunny		
Date sampled :	17 October 2022		
Sampling method :	NZS 4402: 1986 (Coarse)		
Sample description :	Silty SAND with minor gravel	and trace clay	
Sample condition :	As Received	Project No :	6-JRESD.16/6LC
Source :	TSON005	Lab Ref No :	CH9468/4
		Client Ref No :	TSON005

	Test Results	
Client Ref No :	TSON005	
Cone penetration limit :	23	
Plastic limit :	Unable to Roll Threads	
Plasticity index :	NP	
Sample fraction :	Fraction passing 425µm test sieve	
As received water content :	0.2	
Fest Methods		

Test Methods	
Water Content	NZS 4407 : 2015 Test 3.1
Cone Penetration	NZS 4407 : 2015 : Test 3.2
Plastic Limit	NZS 4407 : 2015 : Test 3.3
Plasticity Index	NZS 4407 : 2015 : Test 3.4
Plasticity index	NZ3 4407. 2015. Test 5.4

Date	tested :	22	November	2022
Date	reported :	26	November	2022

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Date :

PF-LAB-053 (09/06/2021)

815 Designation : Laboratory Manager 26 November 2022



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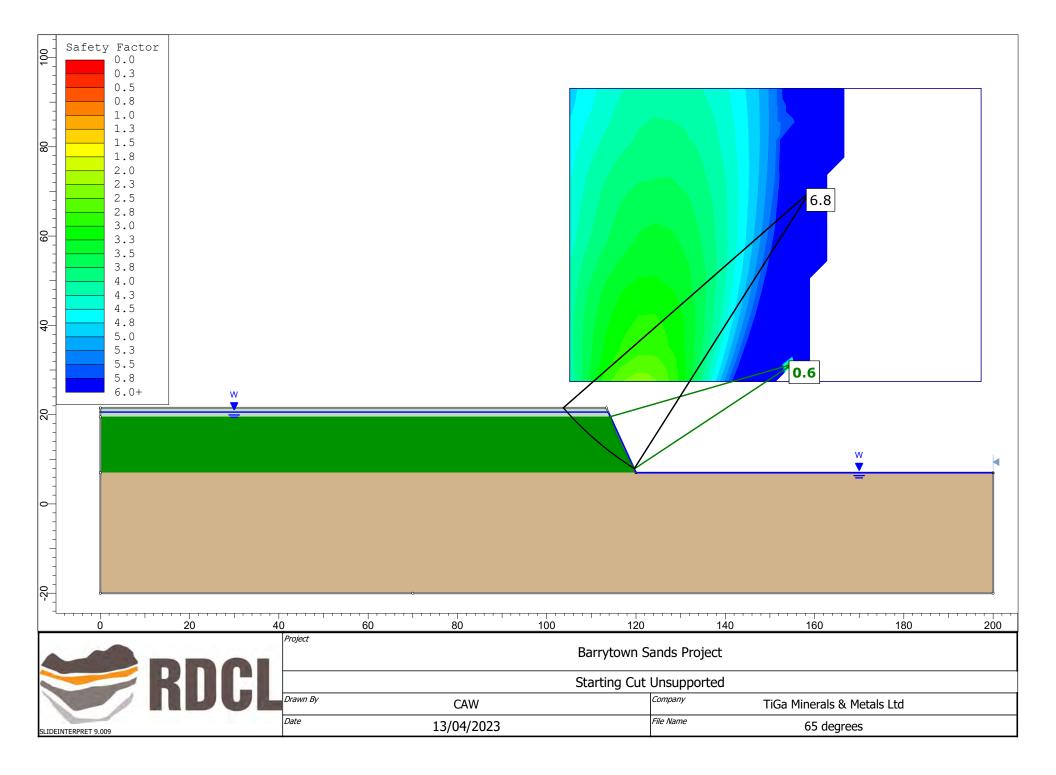
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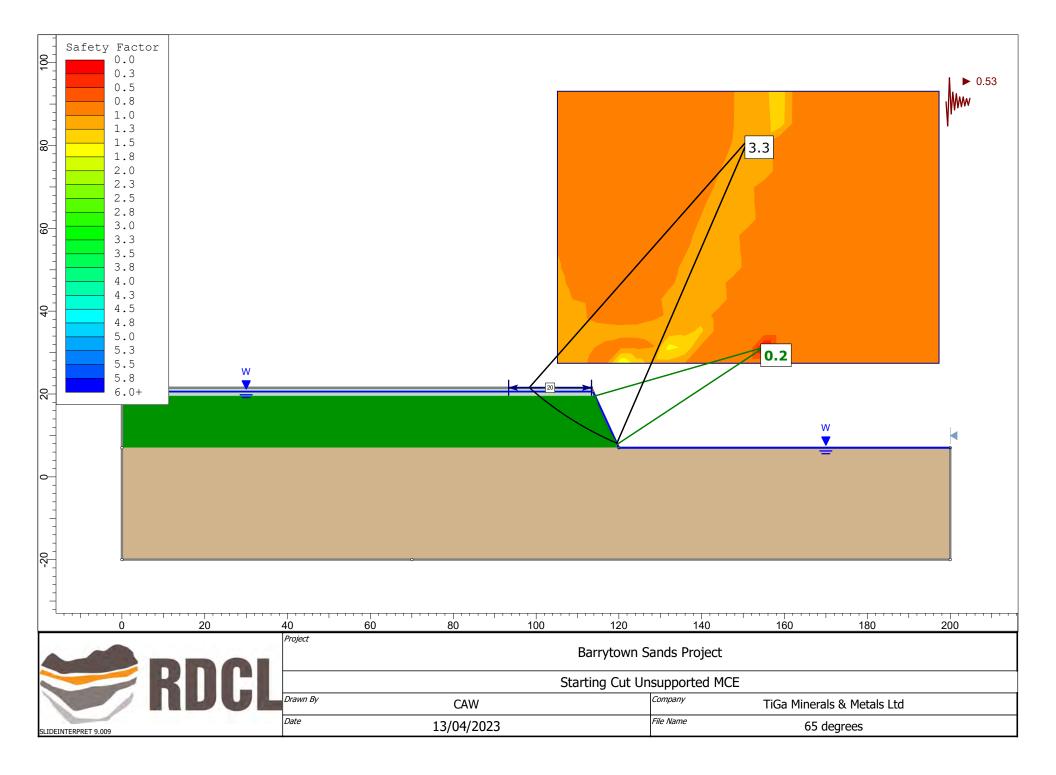
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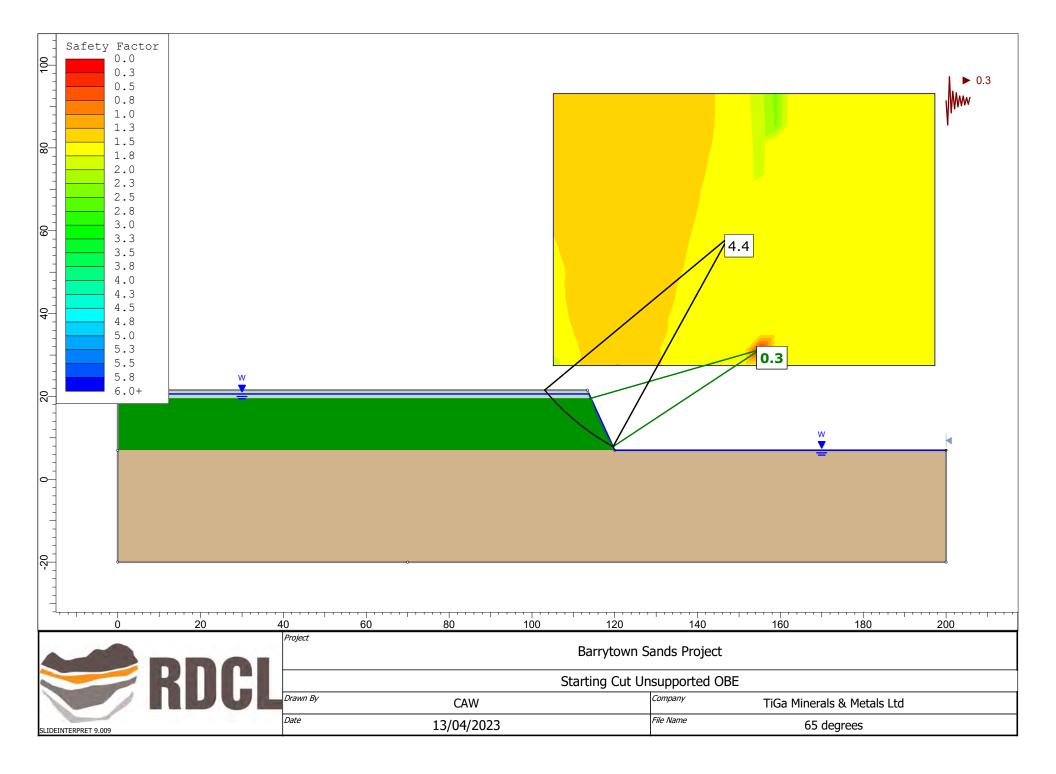
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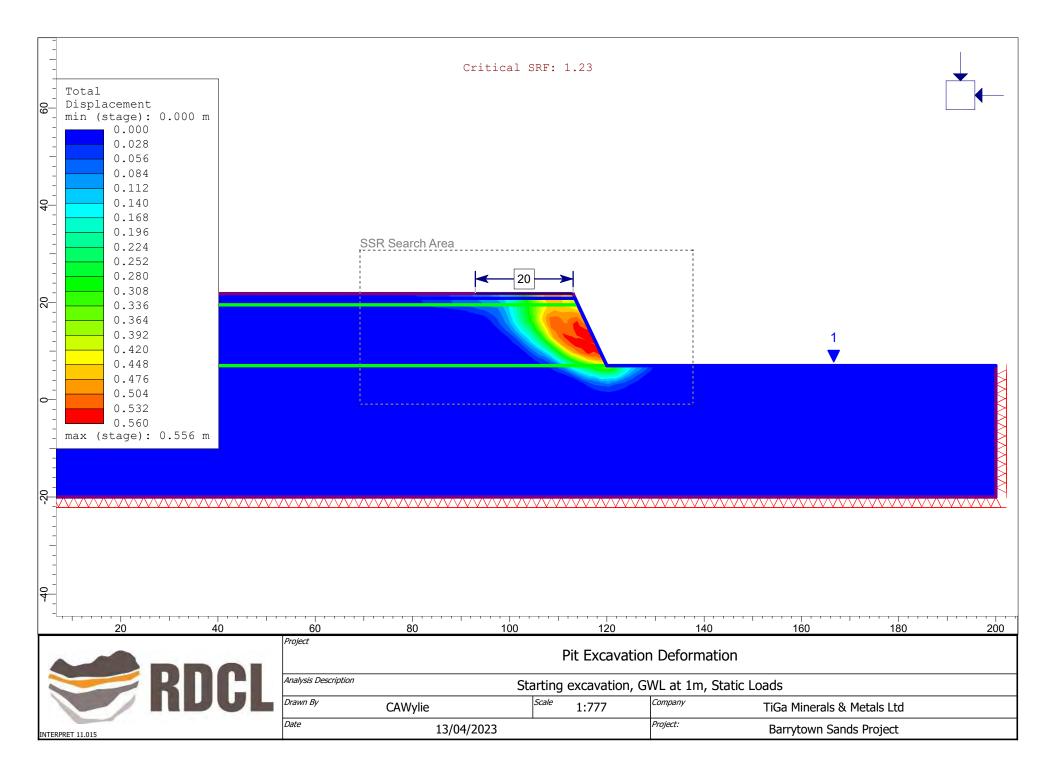
# APPENDIX B STABILITY ANALYSES RESULTS

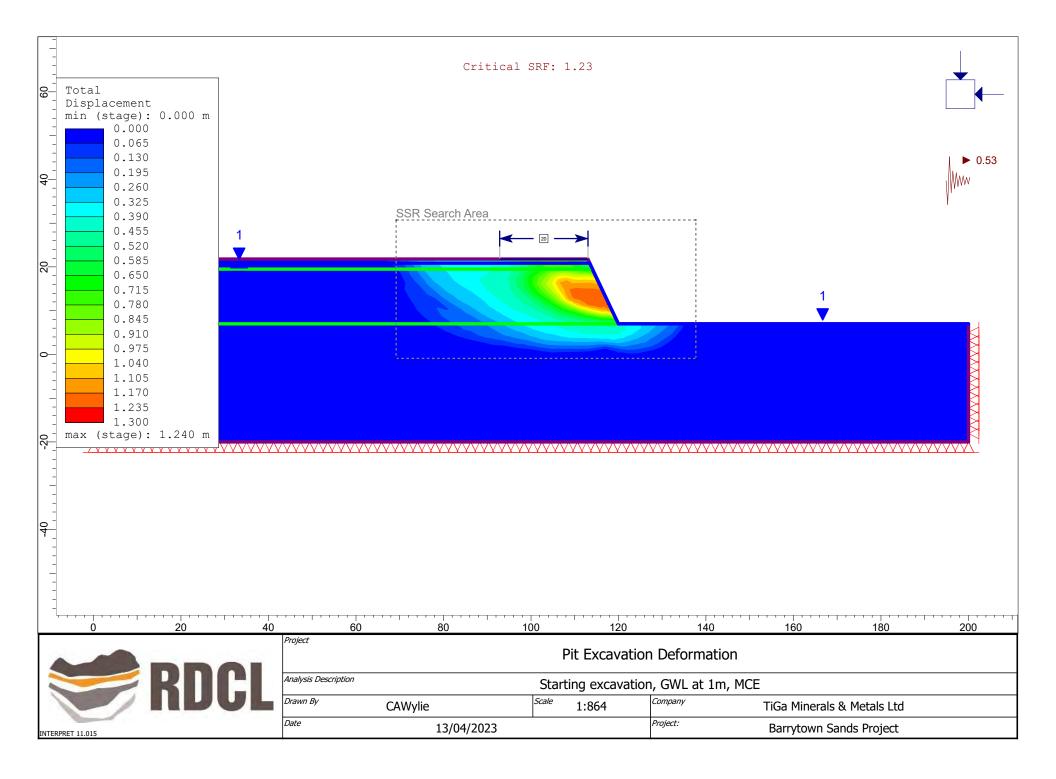


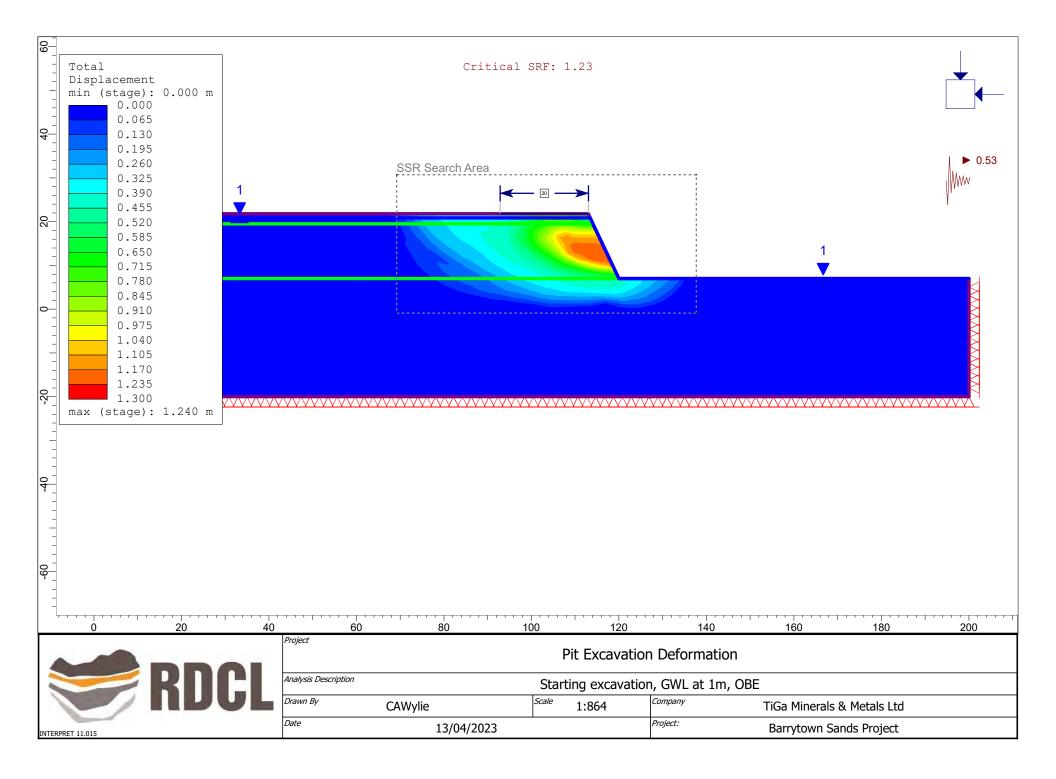


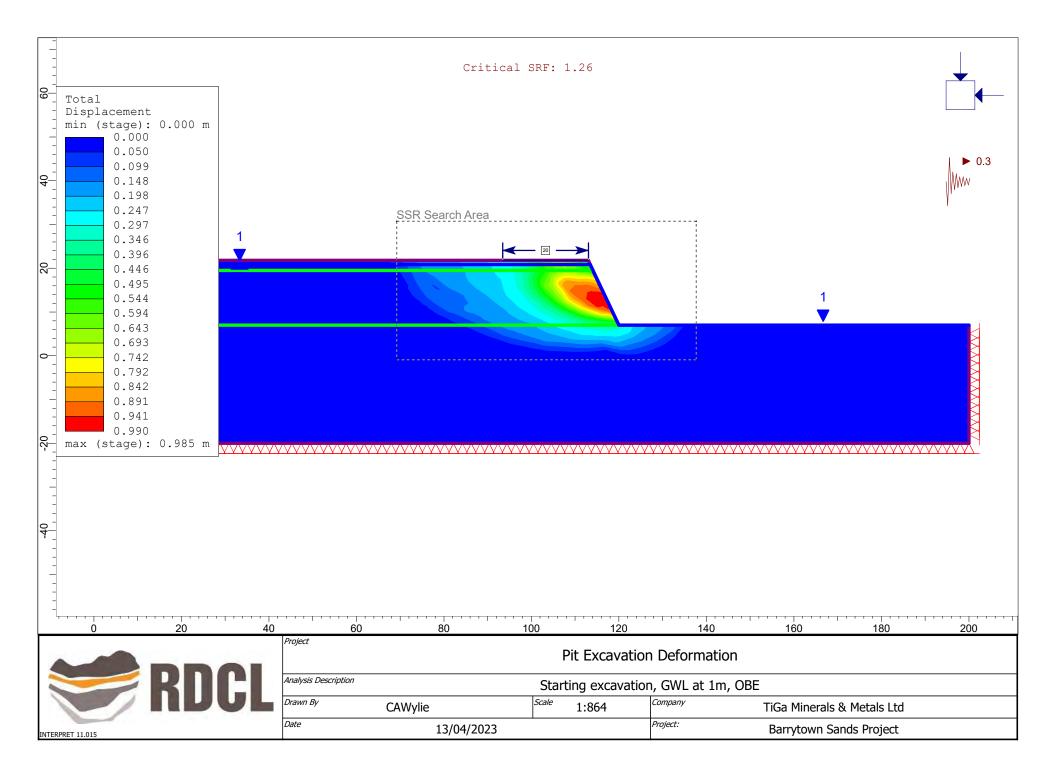


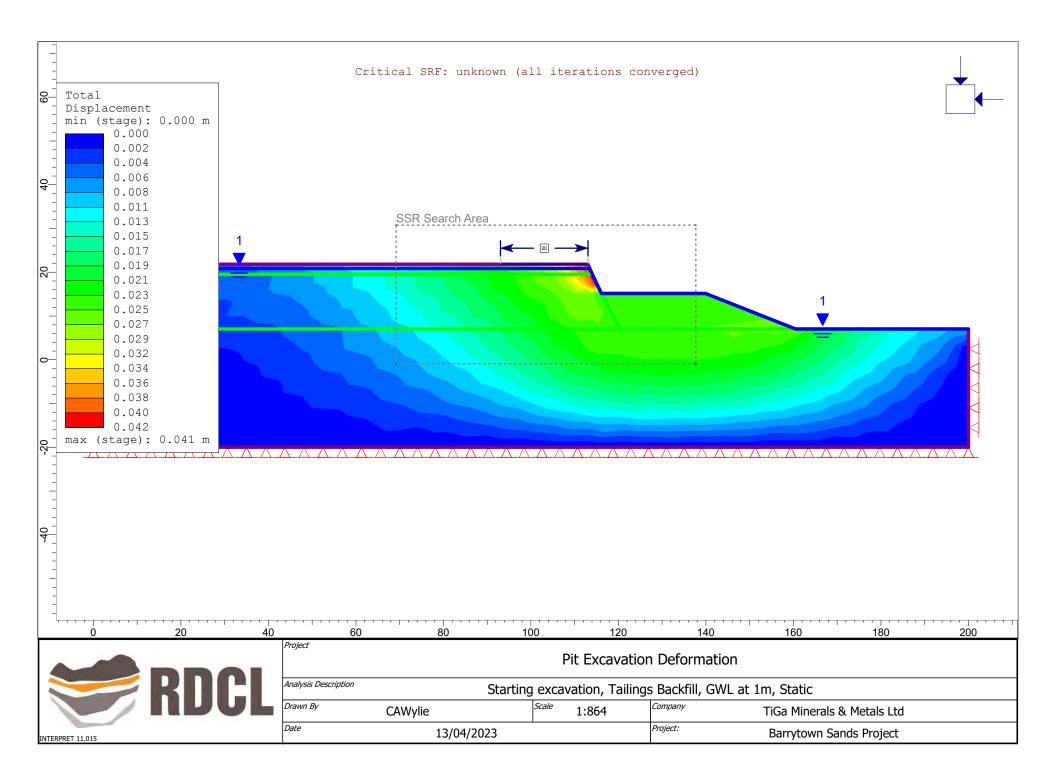


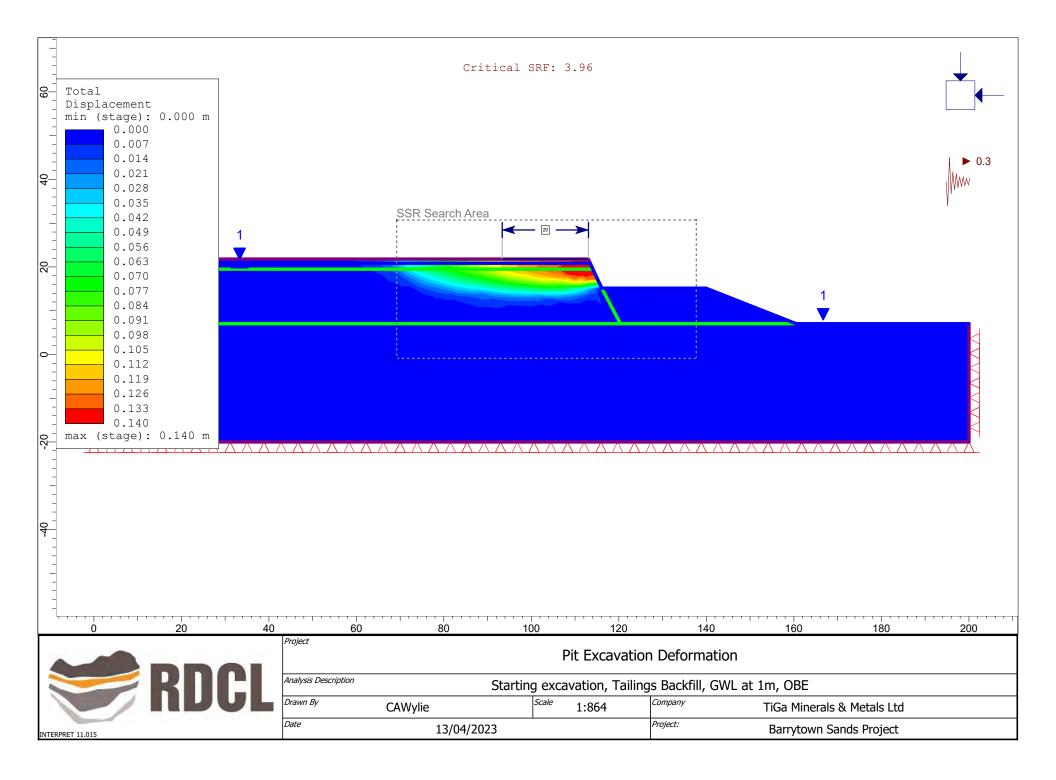


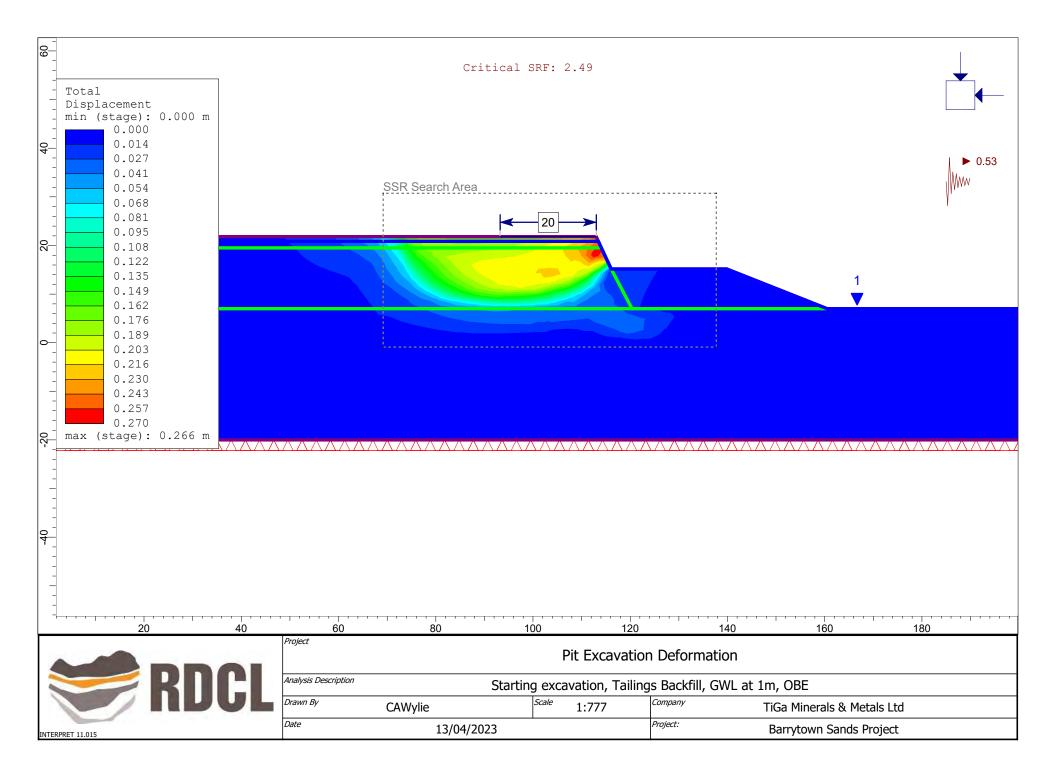












# APPENDIX C POTENTIAL FAILURE MODE ANALYSES



Barrytown Mineral Sands Tailings Storage Facility

		Ra	nking	g / Evaluation o	of Ri	sks		Controls				
Hazard Description	Hazard Type	Consequence		Probability of occurrence		Risk Score		Preventative or intervention measures				
					Co	ompliance a	nd L	egislation International Standards				
Fail to planned, build and operate to Global Tailings Standard (2020) revlevant Priciples	Normal condition	Moderate	3	Possible	3	Moderate	9	Client to confirm criteria for design, and operations				
Fail to consider appropriate standards, local guidance, industry guidelines, and MBIE Guidlines	Normal condition	Major	4	Possible	3	High	12	Confirm requirements and design appropriately				
Resource Consents not granted	Normal condition	Major	4	Possible	3	High	12	Adequatley develop a meaingful study considering NZ RMA and International Tailings Practice requirments				
Knowledge Base inadequate	Normal operation	Major	4	Possible	3	High	12	Meaningfully colate and document use of the information for plan, operation, and design decisions				
Design												
Fail to consider appropriate standards, local guidance, industry guidelines, and MBIE Guidlines	Normal operation	Major	4	Unlikely	2	Moderate	8	Documented methods, transparent references, third party checks.				
Failure to achieve Resource Consent	Normal operation	Major	4	Possible	3	High	12	Design and address Preliminary Site Report as per GTS (2020) Principle 4.2 - " Develop a Prelimnary Design"				
Contaminants in Tailings; total Life cylce	Normal condition	Major	4	Possible	3	High	12	Contaminants excluded from mining and process stream; Full disclosure policy, spillage controls and response plans for environmental spillage approriate monitioring.				
Supernatant Water leads to tailings entrainment in the event of loss of confinement	Normal condition	Major	4	Unlikely	2	Moderate	8	Not Credible Failure Mode. 1) Tailings surface is always < 3m below Natural Ground level (freeboard). 2) Cap tailings for final landform concurrent with mining advance. 3) Surface drains to reduce overland water inflows to pit. 4) Prevent Supernatant Water from operations sumps; for example slurry densification, mining void control, sumps.				
Supernatant Water leads to tailings entrainment in the event of loss of confinement	Extreme condition	Major	4	Unlikely	2	Moderate	8	Not Credible Failure Mode. 1) Tailings surface is always < 3m below Natural Ground level (freeboard). 2) Cap tailings for final landform concurrent with mining advance. 3) Surface drains to reduce overland water inflows to pit. 4) Prevent Supernatant Water from operations sumps; for example slurry densification, mining void control, sumps.				
Loss of confinement due to overtopping	Extreme condition	Major	4	Rare	1	Moderate	4	Not Credible Failure Mode. 1) Tailings surface is always < 3m below Natural Ground level (freeboard). 2) Cap tailings for final landform concurrent with mining advance. 3) Surface drains to reduce overland water inflows to pit. 4) Prevent Supernatant Water from operations sumps; for example slurry densification, mining void control, sumps.				
Loss of confinement due to Slope Instability	Normal condition	Moderate	3	Possible	3	Moderate	9	1) Critical condition is in the initial short term only. Likelihood and consequence decreases as the mine void advances away from the initial cut (boundaries). 2) Design cut slope angle for adequate Factor of Safety considering preliminary geotechnical ground model.				
								1) Critical condition is in the initial short term only. Likelihood and consequence decreases as the mine void advances away from the initial cut (boundaries). 2) Design				

3

3 Possible

Seismic condition Moderate

Loss of confinement due to Slope Instability

9

Moderate

		Author		CA Wylie						
		Company		RDCL						
	Controls		Residual 1							
	Preventative or intervention measures	Consequence		Probabilit occurren	Risk Score					
Legislation International Standards										
Ð	Client to confirm criteria for design, and operations	Moderate		Rare	1	Low	3			
2	Confirm requirements and design appropriately	Moderate	3	Unlikely	2	Low				
2	Adequatley develop a meaingful study considering NZ RMA and International Tailings Practice requirments	Major	4	Unlikely	2	Moderate	8			
2	Meaningfully colate and document use of the information for plan, operation, and design decisions	Major	4	Unlikely	2	Moderate	8			
Design										
3	Documented methods, transparent references, third party checks.	Major	4	Rare	1	Moderate	4			
2	Design and address Preliminary Site Report as per GTS (2020) Principle 4.2 - " Develop a Prelimnary Design"	Major	4	Unlikely	2	Moderate	8			
2	Contaminants excluded from mining and process stream; Full disclosure policy, spillage controls and response plans for environmental spillage approriate monitioring.	Major	4	Unlikely	2	Moderate	8			
3	Not Credible Failure Mode. 1) Tailings surface is always < 3m below Natural Ground level (freeboard). 2) Cap tailings for final landform concurrent with mining advance. 3) Surface drains to reduce overland water inflows to pit. 4) Prevent Supernatant Water from operations sumps; for example slurry densification, mining void control, sumps.	Major	4	Not Credible Failure Mode	0	0	0			
3	Not Credible Failure Mode. 1) Tailings surface is always < 3m below Natural Ground level (freeboard). 2) Cap tailings for final landform concurrent with mining advance. 3) Surface drains to reduce overland water inflows to pit. 4) Prevent Supernatant Water from operations sumps; for example slurry densification, mining void control, sumps.	Major	4	Not Credible Failure Mode	0	0	0			
ļ	Not Credible Failure Mode. 1) Tailings surface is always < 3m below Natural Ground level (freeboard). 2) Cap tailings for final landform concurrent with mining advance. 3) Surface drains to reduce overland water inflows to pit. 4) Prevent Supernatant Water from operations sumps; for example slurry densification, mining void control, sumps.	Major	4	Not Credible Failure Mode	0	0	0			
)	1) Critical condition is in the initial short term only. Likelihood and consequence decreases as the mine void advances away from the initial cut (boundaries). 2) Design cut slope angle for adequate Factor of Safety considering preliminary geotechnical ground model.	Moderate	3	Unlikely	2	Low	6			
)	1) Critical condition is in the initial short term only. Likelihood and consequence decreases as the mine void advances away from the initial cut (boundaries). 2) Design cut slope angle for adequate Factor of Safety considering preliminary geotechnical ground model.	Moderate	3	Rare	1	Low	3			



Barrytown Mineral Sands Tailings Storage Fa	acility						Author		CA Wylie			
							Company		RDCL			
	Ranking / Evaluation of Risks					Controls			Residual 1			
Hazard Description	Hazard Type	Consequence	Probability of occurrence	Risk Score		Preventative or intervention measures	Consequence		Probability of occurrence		Risk Score	
oss of confinement due to Lateral Spread	Normal condition	Major 4	Rare 1	Moderate	4	1) Critical condition is in the initial short term only. Likelihood and consequence decreases as the mine void advances away from the initial cut (boundaries). 2) Tailings backfill is free draining	Moderate	3	Rare	1	Low	ŝ
oss of confinement due to Lateral Spread	Seismic condition	Major 4	Possible 3	High	12	1) Critical condition is in the initial short term only. Likelihood and consequence decreases as the mine void advances away from the initial cut (boundaries). 2) Tailings backfill is free draining	Major	4	Unlikely	2	Moderate	8
oss of confinement due to Piping/Erosion	Normal operation	Major 4	Rare 1	Moderate		Not Credible Failure Mode. 1) Insitu material not prone to piping. 2)Piezometric levels within Tailings always lower than surrounding natural ground.	Major	4	Not Credible Failure Mode	0	0	c
_oss of confinement due to Foundation Failure	Seismic condition	Moderate 3	Rare 1	Low	3	Not Credible Failure Mode. 1) Storage void is in natural ground with foundation level (mine void invert / pit floor) ~8m below natural ground. 2) Tailings insitu density < than natural ground as "heavy mineral" fraction removed by mining.	Moderate	3	Not Credible Failure Mode	0	0	C
ailure to achieve Close Criteria at Completion	Normal condition	Major 4	Possible 3	High	12	1) Clarify on closure expectations. 2) Design for Closure from onset of operations.	Major	4	Rare	1	Moderate	2
	,	· · ·				Operations		-	·			
Failure to recognise risk of unforseen circumstances	Normal operation	Major 4	Possible 3	High	12	Risk Management oversight and systems mainatined over life cycle.	Major	4	Rare	1	Moderate	4
Safety in Design; working in mine void	Normal operation	Moderate 3	Possible 3	Moderate	9	1) Design mining method to reduce exposure, 2) Develop Principle Hazard Management Plan for the situation.	Moderate	3	Rare	1	Low	3
ailings disposal method deviates from original lesign assumption; over life cycle.	Normal operation	Moderate 3	Possible 3	Moderate	9	Risk Management oversight and systems mainatined over life cycle.	Moderate	3	Unlikely	2	Low	e
ailings Slurry Pipeline Spillage	Normal operation	Moderate 3	Possible 3	Moderate	9	Trigger Action Response Plans (TARP's) in place; physical containment infrastructure in place if required, clean up equipment in place if required.	Moderate	3	Unlikely	2	Low	6
Conent conditions not met; over life cycle.	Normal operation	Moderate 3	Possible 3	Moderate	9	Monitoring and review oversight and systems maintained over life cycle.	Moderate	3	Possible	3	Moderate	ę
		· · · · · ·			Em	ergency Response						
Emergency Planning	Extreme condition	Major 4	Possible 3	High	12	TARP's and Emergency Plans established and audited.	Major	4	Unlikely	2	Moderate	8
Recovery from Failure	Extreme condition	Major 4	Possible 3	High	12	Recovery from Catastrophic Failure plan established for life cycle.	Major	4	Unlikely	2	Moderate	8



# Important Information about This Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

### While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

The Geoprofessional Business Association (GBA) has prepared this advisory to help you – assumedly a client representative - interpret and apply this geotechnical-engineering report as effectively as possible. In that way, clients can benefit from a lowered exposure to the subsurface problems that, for decades, have been a principal cause of construction delays, cost overruns, claims, and disputes. If you have questions or want more information about any of the issues discussed below, contact your GBA-member geotechnical engineer. Active involvement in the Geoprofessional Business Association exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project.

# Geotechnical-Engineering Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical-engineering study conducted for a given civil engineer will not likely meet the needs of a civilworks constructor or even a different civil engineer. Because each geotechnical-engineering study is unique, each geotechnicalengineering report is unique, prepared *solely* for the client. *Those who rely on a geotechnical-engineering report prepared for a different client can be seriously misled*. No one except authorized client representatives should rely on this geotechnical-engineering report without first conferring with the geotechnical engineer who prepared it. *And no one – not even you – should apply this report for any purpose or project except the one originally contemplated*.

### **Read this Report in Full**

Costly problems have occurred because those relying on a geotechnicalengineering report did not read it *in its entirety*. Do not rely on an executive summary. Do not read selected elements only. *Read this report in full*.

# You Need to Inform Your Geotechnical Engineer about Change

Your geotechnical engineer considered unique, project-specific factors when designing the study behind this report and developing the confirmation-dependent recommendations the report conveys. A few typical factors include:

- the client's goals, objectives, budget, schedule, and risk-management preferences;
- the general nature of the structure involved, its size, configuration, and performance criteria;
- the structure's location and orientation on the site; and
- other planned or existing site improvements, such as retaining walls, access roads, parking lots, and underground utilities.

Typical changes that could erode the reliability of this report include those that affect:

- the site's size or shape;
- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light-industrial plant to a refrigerated warehouse;
- the elevation, configuration, location, orientation, or weight of the proposed structure;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes – even minor ones – and request an assessment of their impact. *The geotechnical engineer who prepared this report cannot accept responsibility or liability for problems that arise because the geotechnical engineer was not informed about developments the engineer otherwise would have considered.* 

### This Report May Not Be Reliable

Do not rely on this report if your geotechnical engineer prepared it:

- for a different client;
- for a different project;
- for a different site (that may or may not include all or a portion of the original site); or
- before important events occurred at the site or adjacent to it; e.g., man-made events like construction or environmental remediation, or natural events like floods, droughts, earthquakes, or groundwater fluctuations.

Note, too, that it could be unwise to rely on a geotechnical-engineering report whose reliability may have been affected by the passage of time, because of factors like changed subsurface conditions; new or modified codes, standards, or regulations; or new techniques or tools. *If your geotechnical engineer has not indicated an "apply-by" date on the report, ask what it should be*, and, in general, *if you are the least bit uncertain* about the continued reliability of this report, contact your geotechnical engineer before applying it. A minor amount of additional testing or analysis – if any is required at all – could prevent major problems.

### Most of the "Findings" Related in This Report Are Professional Opinions

Before construction begins, geotechnical engineers explore a site's subsurface through various sampling and testing procedures. *Geotechnical engineers can observe actual subsurface conditions only at those specific locations where sampling and testing were performed.* The data derived from that sampling and testing were reviewed by your geotechnical engineer, who then applied professional judgment to form opinions about subsurface conditions throughout the site. Actual sitewide-subsurface conditions may differ – maybe significantly – from those indicated in this report. Confront that risk by retaining your geotechnical engineer to serve on the design team from project start to project finish, so the individual can provide informed guidance quickly, whenever needed.

### This Report's Recommendations Are Confirmation-Dependent

The recommendations included in this report – including any options or alternatives – are confirmation-dependent. In other words, *they are not final*, because the geotechnical engineer who developed them relied heavily on judgment and opinion to do so. Your geotechnical engineer can finalize the recommendations *only after observing actual subsurface conditions* revealed during construction. If through observation your geotechnical engineer confirms that the conditions assumed to exist actually do exist, the recommendations can be relied upon, assuming no other changes have occurred. *The geotechnical engineer who prepared this report cannot assume responsibility or liability for confirmationdependent recommendations if you fail to retain that engineer to perform construction observation*.

### This Report Could Be Misinterpreted

Other design professionals' misinterpretation of geotechnicalengineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer serve as a full-time member of the design team, to:

- confer with other design-team members,
- help develop specifications,
- review pertinent elements of other design professionals' plans and specifications, and
- be on hand quickly whenever geotechnical-engineering guidance is needed.

You should also confront the risk of constructors misinterpreting this report. Do so by retaining your geotechnical engineer to participate in prebid and preconstruction conferences and to perform construction observation.

### **Give Constructors a Complete Report and Guidance**

Some owners and design professionals mistakenly believe they can shift unanticipated-subsurface-conditions liability to constructors by limiting the information they provide for bid preparation. To help prevent the costly, contentious problems this practice has caused, include the complete geotechnical-engineering report, along with any attachments or appendices, with your contract documents, *but be certain to note conspicuously that you've included the material for informational purposes only.* To avoid misunderstanding, you may also want to note that "informational purposes" means constructors have no right to rely on the interpretations, opinions, conclusions, or recommendations in the report, but they may rely on the factual data relative to the specific times, locations, and depths/elevations referenced. Be certain that constructors know they may learn about specific project requirements, including options selected from the report, *only* from the design drawings and specifications. Remind constructors that they may perform their own studies if they want to, and *be sure to allow enough time* to permit them to do so. Only then might you be in a position to give constructors the information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions. Conducting prebid and preconstruction conferences can also be valuable in this respect.

### **Read Responsibility Provisions Closely**

Some client representatives, design professionals, and constructors do not realize that geotechnical engineering is far less exact than other engineering disciplines. That lack of understanding has nurtured unrealistic expectations that have resulted in disappointments, delays, cost overruns, claims, and disputes. To confront that risk, geotechnical engineers commonly include explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely*. Ask questions. Your geotechnical engineer should respond fully and frankly.

### **Geoenvironmental Concerns Are Not Covered**

The personnel, equipment, and techniques used to perform an environmental study – e.g., a "phase-one" or "phase-two" environmental site assessment – differ significantly from those used to perform a geotechnical-engineering study. For that reason, a geotechnicalengineering report does not usually relate any environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated subsurface environmental problems have led to project failures*. If you have not yet obtained your own environmental information, ask your geotechnical consultant for risk-management guidance. As a general rule, *do not rely on an environmental report prepared for a different client, site, or project, or that is more than six months old.* 

# Obtain Professional Assistance to Deal with Moisture Infiltration and Mold

While your geotechnical engineer may have addressed groundwater, water infiltration, or similar issues in this report, none of the engineer's services were designed, conducted, or intended to prevent uncontrolled migration of moisture – including water vapor – from the soil through building slabs and walls and into the building interior, where it can cause mold growth and material-performance deficiencies. Accordingly, *proper implementation of the geotechnical engineer's recommendations will not of itself be sufficient to prevent moisture infiltration*. Confront the risk of moisture infiltration by including building-envelope or mold specialists on the design team. *Geotechnical engineers are not buildingenvelope or mold specialists*.



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